UNIVERSITI TEKNOLOGI MARA

FLEXURAL BEHAVIOUR OF REINFORCED CONCRETE BEAMS STRENGTHENED WITH TEXTILE FINE GRAINED MORTAR

ZALIPAH BINTI JAMELLODIN

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CONFIRMATION BY PANEL OF EXAMINERS

I certify that a Panel of Examiners has met on 14th November 2019 to conduct the final examination of Zalipah Binti Jamellodin in her **Doctor of Philosophy** thesis entitled "Flexural Behaviour of Reinforced Concrete Beams Strengthened with Textile Fine Grained Mortar" in accordance with Universiti Teknologi MARA Act 1976 (Akta 173). The Panel of Examiner recommends that the student be awarded the relevant degree. The Panel of Examiners was as follows:

Junaidah Ariffin, PhD Professor Faculty of Civil Engineering Universiti Teknologi MARA (Chairman)

Kay Dora Abd Ghani, PhD Faculty of Civil Engineering Universiti Teknologi MARA (Internal Examiner)



Andri Kusbiantoro, PhD Associate Professor Faculty of Civil Engineering Technology, Universiti Malaysia Pahang (External Examiner)

J. Revathy, PhD Professor B. S. Abdur Rahman Crescent Institute of Science & Technology (External Examiner)

PROF DR HJH HASLINDA YUSOFF

Dean Institute of Graduates Studies Universiti Teknologi MARA Date: February 2020

ABSTRACT

Recently a new repair and retrofit method have been develop and used to extend the service lives of reinforced concrete (RC) structures. One of the most common reinforcement techniques for RC members involves the use of fibre reinforced polymer (FRP) composites. However, the disadvantage which mainly associated with the use of epoxy resins with high cost, poor performance in high temperature and on wet surface and incompatibility with substrate materials. In an attempt to alleviate the problem arising from the use of epoxies, this research have suggested the replacement with inorganic (mortar) matrix namely fine grained mortar (FGM). Therefore, textile fine grained mortar (TFGM) offers a new innovative technology to strengthen or repair concrete structures. TFGM is comprised of thin layered of FGM and textile reinforcement made of alkali glass resistant (AR glass). Less than 2 mm of mortar thickness is needed between the TFGM layers due to the small maximum grained size of fine sand that is 600 µm. The FGM was designed with three different mix cementitious materials (pozzolans) to determine the optimum compressive strength consisting of fly ash (FA), palm oil fuel ash (POFA) and rice husk ash (RHA) as cement replacement material. The replacement percentage consists of 10%, 20%, 30% and 40% of the weight of the cement. The binder to sand ratio also varies from 1:2, 1:2.5 and 1:3. Studies of using FA as a cement substitute in FGM have been reported by previous researchers. It has demonstrated that inclusion of FA in the FGM improved the strength of the resulted mortar. However, research on use of agriculture wastes namely POFA and RHA as FGM's cement replacement is still lacking. The optimum mix proportion of FGM made of three different pozzolans was selected as a FGM to form composite binder TFGM. Then, the fresh TFGM consisting of two, four, six and eight layers was applied to the surface of plain concrete prism which has no bar reinforcement with dimension of 100 mm x100 mm x 500 mm. Meanwhile, four and eight layers of TFGM have been selected to strengthen RC beams with beam dimensions of 150 mm x 200 mm x 2500 mm. In this research, the lamination method was used to strengthen the plain concrete prism and RC beams. The strengthened plain concrete prism was gone through three point bending test. While for the RC beam specimens were tested under four point bending testing. Finite element analysis using ATENA software was used to verify the experimental work. The result for the FGM mix proportion shows that replacement with 20% FA, 10% POFA and 20% RHA for binder to sand ratio of 1:2 produced an optimum compressive strength. The FGM was then selected and used as a strengthening binder for the plain concrete prism and RC beams. Strengthening with eight layers of TFGM containing POFA on the plain concrete prism increased the load carrying capacity of the latter about 36% compared to the unstrengthened specimen. While for the RC beams with eight layers TFGM containing POFA increased the load carrying capacity about 38% compared to the theoretical load of 30 kN. The load carrying capacity also increased by 33% compared to the unstrengthened specimen of 31 kN. Finally, the prediction of the ATENA software was examined and compared with the experimental results.



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LIST OF SYMBOLS

Symbols

А	Cross section area of specimen
As	Area of steel reinforcement
At	Area of textile reinforcement
F _c	Force in concrete compression
Fs	Force in steel tension
F _t	Force in textile tension
g/m ²	Gram/square milimeter
E _{max}	Maximum strain
δ	Mid-span deflection of a beam
$\Delta_{ m u}$	Ultimate deflection at mid-span of a beam
Δ _y	Deflection of steel at yielding of steel reinforcement
Δ_{\max}	Ultimate deflection at when the material or member yield
Δ_{u}	Ultimate deflection at mid-span of a beam
Р	Load
R _c	Compressive strength (prism)
R _f	Flexural strength (prism)

LIST OF ABBREVIATIONS

Abbreviations

AH	Aluminium Hydroxide
Al ₂ O ₃	Aluminium Hydroxide
ASTM	American Society for Testing and Materials
BET	Brunauer/Emmet/Teller Nitrogen Absorption Test
BS	British Standard
BS EN	Eurocode Standard
CaCO ₃	Calcium Carbonate
CaO	Calcium Oxide
CFRP	Carbon Fibre Reinforced Polymer
DOE	Department of Environment
FA	Fly Ash
FGM	Fine Grained Mortar
FLA-5-11	Type of Strain Gauge for Reinforcement Bar
FLA-5-3	Type of Strain Gauge for Shear Link
FRP	Fibre Reinforced Polymer
K ₂ O	Potassium Oxide
LOI	Loss of Ignition
LVDT	Linear Variable Differential Transformer

REFERENCES

- ASTM International C618 (2015). Standard specification for coal fly ash and raw or calcined natural pozzolan for use in concrete. West Conshohocken.
- Babaeidarabad, S., Loreto, G. & Nanni, A. (2014). Flexural strengthening of RC beams with an externally bonded fabric-reinforced cementitious matrix. *Journal of Composites for Construction*, 18(5), pp. 1–12.
- Banholzer, B., Brockmann, T. & Brameshuber, W. (2006). Material and bonding characteristics for dimensioning and modelling of textile reinforced concrete (TRC) elements. *Materials and Structures*, 39(8), pp. 749–763.
- Barhum, R. & Mechtcherine, V. (2012). Influence of short dispersed and short integral glass fibres on the mechanical behaviour of textile-reinforced concrete. *Materials and Structures*, 46(4), pp. 557–572.
- Bendapudi, S. & Saha, P. (2011). Contribution of fly ash to the properties of mortar and concrete. *International Journal Earth Science Engineering*, 04(06), pp.1017– 1023.
- Bentur, A. & Mindess, S. (2006). Fibre Reinforced Cementitious Composite, London: Taylor & Francis Group London
- Bhutta, M. A. R., Hafizah, N. A. K., Jamaludin, M. Y., Warid, M. H., Ismail, M. & Azman, M. (2013). Strengthening reinforced concrete beams using Kenaf fiber reinforced polymer composite laminates. *Third International Conference on Sustainable Construction Materials and Technologies*, 19-21 August 2013, Kyoto Research Park, Kyoto, Japan.
- Biricik, H. & Sarier, N. (2014). Comparative study of the characteristics of nano silica silica fume and fly ash incorporated cement mortars. *Materials Research*, 17(3), pp. 570–582.
- Bouzoubaa, N., Zhang, M.H. & Malhotra, V.M. (2001). Mechanical properties and durability of concrete made with high-volume fly ash blended cements using a coarse fly ash. *Cement and Concrete Research*, 31, pp.1393–1402.
- Brameshuber, W. (2006). Textile Reinforced Concrete, RILEM Report.
- Brockmann, T. (2005). Mechanical and Fracture Mechanical Properties of Fine Grained Concrete for Textile Reinforced Composites. Thesis Dissertation. RWTH Aachen University, Germany.

- Brückner, A., Ortlepp, R. & Curbach, M. (2006). Textile reinforced concrete for strengthening in bending and shear. *Materials and Structures*, 39(8), pp.741– 748.
- Brückner, A., Ortlepp, R. & Curbach, M. (2007). Anchoring of shear strengthening for T-beams made of textile reinforced concrete (TRC). *Materials and Structures*, 41(2), pp.407–418.
- BS EN 196-1. (2005). Methods of testing cement Part 1: Determination of strength, British Standard International Group.
- BS EN 196-3. (2005). Methods of testing cement Part 2: Determination of setting times and soundness, British Standard International Group.
- BS EN 196-6. (2005). Methods of testing cement Part 6: Determination of fineness, British Standard International Group.

BS EN 1992-1-1. (2004). Eurocode 2 : Design of concrete structures. British Standard.

BS EN 12620. (2013). Aggregates for Concrete. British Standard International Group.

- Butler, M., Mechtcherine, V. & Hempel, S. (2009). Experimental investigations on the durability of fibre–matrix interfaces in textile-reinforced concrete. *Cement and Concrete Composites*, 31(4), pp. 221–231.
- Butler, M., Mechtcherine, V. & Hempel, S. (2010). Durability of textile reinforced concrete made with AR glass fibre: effect of the matrix composition. *Materials and Structures*, 43(10), pp. 1351–1368.
- Cervenka, V., Jendele, L. & Cervenka, J. (2015). ATENA Program Documentation Part 4-1. *Cervenka Consulting*, Prague Czech Republic.
- Chandara, C., Sakai, E., Azizli, K. A. M., Ahmad, Z. A. & Hashim, S. F. S. (2010). The effect of unburned carbon in palm oil fuel ash on fluidity of cement pastes containing superplasticizer. *Construction and Building Materials*, 24(9), pp. 1590–1593.
- Cheerarot, R. & Jaturapitakkul, C. (2004). A study of disposed fly ash from landfill to replace Portland cement. *Waste Management*, 24, pp. 701–709.
- Chindaprasirt, P., Homwuttiwong, S. and Sirivivatnanon, V. (2004). Influence of fly ash fineness on strength, drying shrinkage and sulfate resistance of blended cement mortar, *Cement and Concrete Research*, 34(7), pp. 1087–1092



- Chindaprasirt, P., Jaturapitakkul, C. & Sinsiri, T. (2005). Effect of fly ash fineness on compressive strength and pore size of blended cement paste. *Cement and Concrete Composites*, 27(4) pp. 425–428.
- Chindaprasirt, P., Jaturapitakkul, C. & Sinsiri, T. (2007a). Effect of fly ash fineness on microstructure of blended cement paste. *Construction and Building Materials*, 21, pp. 1534–1541.
- Chindaprasirt, P., Homwuttiwong, S. and Jaturapitakkul, C. (2007b). Strength and water permeability of concrete containing palm oil fuel ash and rice husk–bark ash. *Construction and Building Materials*, 21(7) pp. 1492 – 1499.
- Chindaprasirt, P., Rukzon, S. & Sirivivatnanon, B. (2008). Resistance to chloride penetration of blended Portland cement mortar containing palm oil fuel ash, rice husk ash and fly ash. *Construction and Building Materials*, 22, pp. 932–938.
- Chindaprasirt, P. & Rukzon, S., (2008). Strength, porosity and corrosion resistance of ternary blend Portland cement, rice husk ash and fly ash mortar. *Construction and Building Materials*, 22(8), pp. 1601–1606.
- Choi, S., Lee, S. & Monteiro, P. (2012). Effect of fly ash fineness on temperature rise, setting, and strength development of mortar. *Journal of Materials in Civil Engineering*, 4(5), pp. 499–505.
- Contamine, R., Si Larbi, A. & Hamelin, P. (2011). Contribution to direct tensile testing of textile reinforced concrete (TRC) composites. *Materials Science and Engineering*, 528(29–30), pp. 8589–8598.
- Curbach, M. & Schreerer, S., 2011. Concrete Light Possibilities and visions. In Proceedings FIB Symposium. 8 – 10 June 2011, Prague, Czech Republic. pp. 29–44.
- D'Ambrisi, A., Feo, L. & Focacci, F. (2013). Experimental analysis on bond between PBO-FRCM strengthening materials and concrete. *Composites Part B: Engineering*, 44(1), pp. 524–532.
- D'Ambrisi, A. & Focacci, F. (2011). Flexural Strengthening of RC Beams with Cement-Based Composites. *Journal of Composites for Construction*, 15 (October), pp. 707–720.
- Dolatabadi, M.K. Janetzko, S., Gries, T., Kang, B. G. and Sander, A. (2010). Permeability of AR-glass fibers roving embedded in cementitious matrix. *Materials and Structures*, 44(1), pp. 245–251.

- Ebead, U., Shrestha K. C., Afzal, M. S., El Refai, A. & Nanni, A. (2017). Effectiveness of Fabric-Reinforced Cementitious Matrix in Strengthening Reinforced Concrete Beams. Journal of Composites for Construction, 21(2), 04016084
- El-Dakroury, A. & Gasser, M. S. (2008). Rice husk ash (RHA) as cement admixture for immobilization of liquid radioactive waste at different temperatures. *Journal of Nuclear Materials*, 381(3), pp. 271–277.
- El-Sherif, H. E., Wakjira, T. G. & Ebead, U. (2020). Flexural strengthening of reinforced concrete beam using hybrid near-surface embedded/externally bonded fabric-reinforced cementitious matrix. *Construction and Building Materials*, 238: 117748
- Elkhadiri, I., Diouria, A., Boukharia, A., Arideb, J. & Puertasc, F. (2002). Mechanical behaviour of various mortars made by combined fly ash and limestone in Moroccan Portland cement. *Cement and Concrete Research*, 32(10), pp. 1597–1603.
- Elsanadedy, H. M., Almusallam, T. H., Alsayed, S. H. and Al-Salloum, Y. A. (2013) Flexural strengthening of RC beams using textile reinforced mortar – Experimental and numerical study. *Composite Structures*, 97, pp. 40–55.
- Ephraim, M.E., Akeke, G.A. & Ukpata, J.O. (2012). Compressive strength of concrete with rice husk ash as partial replacement of ordinary Portland cement. *Journal of Research Engineering*, 1, pp. 32–36.
- Erdogdu, K. & Tucker, P. (1998). Effect of fly ash particles size on strength of Portland cement fly ash mortars. *Cement and Concrete Research*, 28(9), pp. 1217–1222.
- Fangueiro, R. (2001), Fibrous and Composite Materials for Civil Engineering Applications. Elsevier.
- Farzadnia, N., Noorvand, H., Yasin, A. M. & Aziz, F. A. N. (2015). The effect of nano silica on short term drying shrinkage of POFA cement mortars. *Construction* and Building Materials. 95, pp. 636–646
- Funke, H., Gelbrich, S. & Ehrlich, A. (2013). Development of a new hybrid material of textile reinforced concrete and glass fibre reinforced plastic 2. *Procedia Materials Science*, pp.113-110
- Ganesan, K., Rajagopal, K. & Thangavel, K. (2008). Rice husk ash blended cement: Assessment of optimal level of replacement for strength and permeability



properties of concrete. *Construction and Building Materials*, 22(8), pp.1675–1683.

- Gencoglu, M. & Mobasher, B. (2005). Monotonic and cyclic flexural behavior of plain concrete beams strengthened by fabric-cement based composites. *Structural Engineering, Mechanics and Computation*, pp. 1961–1966.
- Givi, A. N., Rashid, S. A., Aziz, F. N. A., Salleh, M. A. M. (2010). Assessment of the effects of rice husk ash particle size on strength, water permeability and workability of binary blended concrete. *Construction and Building Materials*, 24(11), pp. 2145–2150.
- Guo, R., Hu, W., Li, M. & Hino, S. (2020). Study on the flexural strengthening effect of RC beams reinforced by FRP grid with PCM shotcrete. *Composite Structures*.
- Hafizah, N. A. K., Bhutta, M. A. R., Jamaludin M. Y., Warid, M. H. & Ismail, M. (2014). Kenaf Fiber Reinforced Polymer Composites for Strengthening RC Beams. *Journal of Advanced Concrete Technology*, 12(6), pp. 167–177.
- Hartig, J., Häußler-Combe, U. & Schicktanz, K. (2008). Influence of bond properties on the tensile behaviour of Textile Reinforced Concrete. *Cement and Concrete Composites*, 30(10), pp. 898–906.
- Hartig, J., Jess, F., Schicktanz, K. & Häußler-Combe, U. (2011). Influence of experimental setups on the apparent uniaxial tensile load-bearing capacity of Textile Reinforced Concrete specimens. *Materials and Structures*, 45(3), pp. 433–446.
- Häußler-Combe, U. & Hartig, J. (2007). Bond and failure mechanisms of textile reinforced concrete (TRC) under uniaxial tensile loading. *Cement and Concrete Composites*, 29(4), pp. 279–289
- Hegger, J., Will, N., Bruckermann, O. & Voss, S. (2006). Load–bearing behaviour and simulation of textile reinforced concrete. *Materials and Structures*, 39(8), pp. 765–776.
- Hegger, J. & Voss, S. (2008). Investigations on the bearing behaviour and application potential of textile reinforced concrete. *Engineering Structures*, 30(7), pp. 2050–2056.
- Hsu, S., Chi, M. & Huan, R. (2018). Effect of fineness and replacement ratio of ground fly ash on properties of blended cement mortar. *Construction and Building Materials*, 176, pp. 250-258.

- Isaia, G. C. (2003). Physical and pozzolanic action of mineral additions on the mechanical strength of high-performance concrete. *Cement and Concrete Composite*, 25, pp. 69–76.
- Islam, M. M. & Islam, M. S. (2012). Strength Behaviour of Mortar Using Fly Ash as Partial Replacement of Cement. *Concrete Research Letters*, 1(3), 2010.
- Jabr, A., El-Ragaby, A. and Ghrib, F. (2017). Effect of the Fiber Type and Axial Stiffness of FRCM on the Flexural Strengthening of RC Beams. *Fibers*, 5(1), pp. 2.
- Jamil, A., Khan, M. N. N., Karim, M. R., Kaish, A. B. M. A. & Zain, M. F. M. (2016). Physical and chemical contributions of rice husk ash on the properties of mortar. *Construction and Building Materials*, 128, pp. 185–98.
- Jaya, R. P., Bakar, B. H. A., Johari, M. A. M. & Ibrahim, M. H. W. (2011). Strength and permeability properties of concrete containing rice husk ash with different grinding time. *Central European Journal of Engineering*, 1, pp. 103-112.
- Jaturapitakkul, C., Tangpagasit, J., Songmue, S. & Kiattikomol, K. (2011). Filler effect and pozzolanic reaction of ground palm oil fuel ash. *Construction and Building Materials*, 25(11), pp. 4287–4293.
- Johari, M. A. M., Zeyad, A. M., Bunnori, N. M. & Ariffin, K. S. (2012). Engineering and transport properties of high-strength green concrete containing high volume of ultrafine palm oil fuel ash. *Construction and Building Materials*, 30, pp. 281–288
- Karim, M., Zain, M. & Jamil, M. (2012). Strength of Mortar and Concrete as Influenced by Rice Husk Ash: A Review. World Applied Sciences Journal, 19(10), pp. 1501–1513.
- Karim, M., Hossain, M. M., Khan, M. N. N., Zain, M. F. M, Jamil, M. & Lai, F. C. (2014). On the utilization of pozzolanic wastes as an alternative resource of cement. *Materials*, 7, pp. 7809–7827.
- Kartini, K., Mahmud, H. B. & Hamidah, M. S. (2010). Absorption and permeability performance of Selangor rice husk ash blended grade 30 concrete. *Journal of Engineering Science and Technology*, 5(1) pp. 1–16.
- Kocak, Y. & Nas, S. (2014). The effect of using fly ash on the strength and hydration characteristics of blended cements. *Construction and Building Materials*. 73, pp. 25-32.

- Kong, K., Mesticau, Z., Michel, M., Si Larbi, A. & Junes, A. (2017). Comparative characterization of the durability behaviour of textile-reinforced concrete (TRC) under tension and bending. *Composite Structures*, 179, pp. 107–123.
- Koutas, L. N., Tetta, Z., Bournas, D. A. & Triantafillou, T. C. (2018). Strengthening of concrete structures with Textile Reinforced Mortars: State-of-the-art Review. *Journal of Composites for Construction*. 23(1), pp. 1-20
- Kroehong, W., Theerawat, S., Chai, J. & Chindaprasirt, P. (2011). Effect of palm oil fuel ash fineness on the microstructure of blended cement paste. *Construction and Building Materials*, 25(11), pp. 4095–4104.
- Kuroda, M., Watanabe, T. & Terashi, N. (2000). Increase of bond strength at interfacial transition zone by the use of fly ash. *Cement and Concrete Research*, 30, pp.253–258.
- Li, G. & Wu, X. (2015). Influence of fly ash and its mean particle size on certain engineering properties of cement composite mortars. *Cement and Concrete Research*, 35, pp. 1128–1134.
- Lim, N. H. A. S., Ismail, M. A., Lee H. S., Hussin M. W., Sam A. R. M. & Samadi M. (2015). The effects of high volume nano palm oil fuel ash on microstructure properties and hydration temperature of mortar. *Construction and Building Materials*, 93, pp 29–34.
- Mechtcherine, V. (2013). Novel cement-based composites for the strengthening and repair of concrete structures. *Construction and Building Materials*, 41, pp. 365–373.
- Mumenya, S. W., Tait, R. B. & Alexander, M. G. (2010). Evaluation of toughness of textile concrete. *Materials and Structures*, 44(1), pp. 279–89.
- Neville, A.M., 2011. Properties of Concrete 5th edition. England: Pearson Education Limited.
- Noorvand, H., Ali, A. A., Demirboga, R., Noorvand, H. & Farzadnia, N. (2013). Physical and chemical characteristics of unground palm oil fuel ash cement mortars with nanosilica. *Construction and Building Materials*, 48, pp. 1104–1113.
- Ombres, L. (2011). Flexural analysis of reinforced concrete beams strengthened with a cement based high strength composite material. *Composite Structures*, 94(1), pp. 143–155.

- Ortlepp, R., Hampel, U. & Curbach, M. (2006). A new approach for evaluating bond capacity of TRC strengthening. *Cement and Concrete Composites*, 28(7), pp. 589–597.
- Ortlepp, R. (2011). Anchorage length for textile reinforced concrete. *International Journal of Environmental Protection*, 1(3), pp. 43–48.
- Pam, H. J., Kwan, A. K. H., & Islam, M. S. (2001). Flexural strength and ductility of reinforced normal and high-strength concrete beams. *In proceedings of the Institution of Civil Engineers*, 381–389.
- Papanicolaou, C. G., Triantafillou, T. C, Papathanasiou, M. & Karlos, K. (2007). Textile reinforced mortar (TRM) versus FRP as strengthening material of URM walls: out-of-plane cyclic loading. *Materials and Structures*, 41(1), pp. 143–157.
- Park, R. (1988). Ductility evaluation from laboratory and analytical testing. In Proceeding of Ninth World Conference on Earthquake Engineering, Tokyo Japan, August, 605–616.
- Peled, A., (2007). Confinement of damaged and nondamaged structural concrete with FRP and TRC sleeves. *Journal of Composites for Construction*, 11(5), pp.514–522.
- Rafieizonooz, M., Mirza, J., Salim, M. R., Hussin, M. W. & Khankhaje, E. (2016).
 Investigation of coal bottom ash and fly ash in concrete as replacement for sand and cement. *Construction and Building Materials*, 116, pp. 15–24.
- Ramezanianpour, A., Mahdi, A. M. & Ahmadibeni G. H. (2009). The effect of rice husk ash on mechanical properties and durability of sustainable concretes. *International Journal of Civil Engneering*, 7(2), pp. 83–91.
- Ramezanianpour, A. A., Pourbeik, P., Mahdikhani, M. & Moodi, F (2010). Mechanical properties and durability of concretes containing rice husk ash as supplementary cementing material. In 2nd International Conference on Sustainable Construction Materials and Technologies, Italy.
- Ranjbar, N., Mehrali, M., Behnia, A., Alengaram, U. J. & Jumaat, M. Z. (2014). Compressive strength and microstructural analysis of fly ash / palm oil fuel ash based geopolymer mortar under elevated temperatures. *Construction & Building Materials*, 65, pp. 114–121.

- Raoofa, S. M., Lampros, N., Koutas, C. & Bournas, D. A. (2017). Textile-reinforced mortar (TRM) versus fibre-reinforced polymers (FRP) in flexural strengthening of RC beams. *Construction and Building Materials*, 151, pp. 279–291.
- Sata, V., Jaturapitakkul, C & Kiattikomol, K. (2007). Influence of pozzolan from various by-product materials on mechanical properties of high-strength concrete. *Construction and Building Materials*, 21, pp. 1589–1598.
- Sata, V., Tangpagasit, J., Jaturapitakkul, C. & Chindaprasirt, P. (2012). Effect of W/B ratios on pozzolanic reaction of biomass ashes in portland cement matrix. *Cement and Concrete Composites*, 34(1), pp. 94–100.
- Schladitz, F., Lorenz, E., Jesse, F. & and Curbach, M. (2009). Strengthening of a Barrelshaped roof using textile reinforced concrete. *In Proceedings of 33rd IABSE 2009 September 9-11, Bangkok, Thailand*. pp. 416–422.
- Schladitz, F. & Curbach, M. (2012). Bending load capacity of reinforced concrete slabs strengthened with textile reinforced concrete. *Engineering Structures*, 40, pp. 317–326
- Shaikh, F. U. A., Supit, S. W. M. & Sarker, P. K. (2014). A study on the effect of nano silica on compressive strength of high volume fly ash mortars and concretes. *Materials & Design*, 60, pp. 433–442.
- Si Larbi, A., Contamine, R., Ferrier, E. & Hamelin, P. (2010). Shear strengthening of RC beams with textile reinforced concrete (TRC) plate. *Construction and Building Materials*, 24(10), pp. 1928–1936.
- Sneed, L. H., Verre, S., Carloni, C. & Ombress, L. (2016). Flexural behavior of RC beams strengthened with steel-FRCM composite. *Engineering Structures*, 127 pp. 686–699.
- Sujivorakul, C., Jaturapitakkul, C. & Taotip, A. (2011). Utilization of fly ash, rice husk ash, and palm oil fuel ash in glass fiber–reinforced concrete. *Journal of Materials in Civil Engineering*, 23 (9), pp. 1281–1288.
- Tahir, M.A. & Sabir, M. (2005). A study on durability of fly ash-cement mortars a study on durability of fly ash cement mortars. 30th Conference on Our World in Concrete & Structures, Singapore.
- Tangchirapat, W., Saeting, T., Jaturapitakkul, C., Kiattikomol, K. and Siripanichgorn, A. (2007). Use of Waste Ash from Palm Oil Industry in Concrete. Waste Management, 27, pp.81–88.

- Tangchirapat, W., Jaturapitakkul, C. & Chindaprasirt, P. (2009a). Use of palm oil fuel ash as a supplementary cementitious material for producing high-strength concrete. *Construction and Building Materials*, 23(7), pp. 2641–2646.
- Tangchirapat, W., Jaturapitakkul, C. & Kiattikomol, K., (2009b). Compressive strength and expansion of blended cement mortar containing palm oil fuel ash. *Journal* of Materials in Civil Engineering, 21 (8), pp. 426–431.
- Tay, J. & Show, K. (1995). Use of ash derived from oil-palm waste incineration as a cement replacement material. *Resources, Conservation and Recycling*, 13, pp. 27 – 36
- Teng, J. G, Chen, J. F. & Smith, S. T. (2002). FRP Strengthened RC structures. John Wiley
- Toutanji, H., Delattec, N., Aggounb, S., Duvalb, R. & Danson, A. (2004). Effect of supplementary cementitious materials on the compressive strength and durability of short-term cured concrete. *Cement and Concrete Research*, 34(2), pp. 311–319.
- Toutanji, H., Zhao, L. & Zhang, Y. (2006). Flexural behavior of reinforced concrete beams externally strengthened with CFRP sheets bonded with an inorganic matrix. *Engineering Structures*, 28, pp. 557–566.
- Triantafillou, T. & Papanicolaou, C. (2006). Shear strengthening of reinforced concrete members with textile reinforced mortar (TRM) jackets. *Materials and Structures*, 39(285), pp. 93–103.
- Turk, K. (2012). Viscosity and hardened properties of self-compacting mortars with binary and ternary cementitious blends of fly ash and silica fume. *Construction* and Building Materials, 37, pp. 326–334.
- Valter, D. (2001). Durability of FRP reinforcement in concrete: Literature review and experiments. Thesis for Chalmers University of Technology. Domone.
- Wang, A., Zhang, C. & Sun, W. (2003). Fly ash effects: I. The morphological effect of fly ash. *Cement and Concrete Research*, 33(12), pp. 2023–2029.
- Weiland, S., Ortlepp, R., Hauptenbuchner, B. & Curbach, M. (2008). Textile reinforced concrete for flexural strengthening of RC-structures - Part 2: Application on a concrete shell. *American Concrete Institute*, 251, pp. 42–68.

- Weiland, S. & Lorenz, E. (2012). Flexural strengthening of RC-structures by textile reinforced concrete in practical application. Taylor & Francis Group, LLC, pp. 783–788.
- Williams, P. N., Lundgren, K., Holger, W. & Katarina, M. (2014). Sustainable potential of textile-reinforced concrete. *Journal of Materials in Civil Engineering*, 27(7), pp. 334-341
- Wong, Y. L., Lam, L., Poon, C. S. & Zhou, F. P. (1999). Properties of fly ash-modified cement mortar-aggregate interfaces. *Cement and Concrete Research*, 29, pp. 1905–1913.
- Xu, S. & Krüger, M. (2004). Bond characteristics of carbon, alkali resistant glass, and aramid textiles in mortar. *Journal of Materials in Civil Engineering*, 16(4), pp. 356–364.
- Xu, W., Lo, T.Y. & Memon, S.A. (2012). Microstructure and reactivity of rich husk ash. *Construction and Building Materials*, 29, pp. 541–547.

AUTHOR'S PROFILE



Zalipah Binti Jamellodin completed her PhD in Civil Engineering at the Faculty of Civil Engineering, Universiti Teknologi MARA. She obtained her Master of Engineering (Structure) in 2007 and Bachelor of Civil Engineering (Hons) in 2000 from Universiti Teknologi Malaysia at Faculty of Civil Engineering, Universiti Teknologi Malaysia. Currently she is working at Faculty of Civil and Built Environment, Universiti Tun Hussein Onn Malaysia (UTHM) since 2006. Experienced as a structural engineer in two private construction companies for 4 years before working at UTHM. The area of expertise focuses on structural and materials engineering such as structural repairs and materials modification.



LIST OF PUBLICATION:

- Jamellodin, Z. Saman, H. M., Adnan, S. H., Mohammad, N. S. and Yusof, W. Y. W. (2014). Compressive and Flexural Strength of Fine Grained Mortar Containing Rice Husk Ash: A Review. Advanced Materials Research. Vol.1051 : pp. 757-762.
- Jamellodin, Z. Saman, H. M., Adnan, S. H., Mohammad, N. S. and Yusof, W. Y. W. (2015). Strength Development of Fine Grained Mortar Containing Fly Ash and Rice Husk Ash. *Applied Mechanics and Materials*. pp. 752-753.

- Yusof, W. Y. W., Adnan, S. H., Jamellodin, Z. and Mohammad, N. S. (2015). Strength Development of Fine Grained Mortar Containing Palm Oil Fuel Ash as a Partial Cement Replacement. *Applied Mechanics and Materials*; Vol 773-774, pp 964-968.
- Mohammad, N. S., Adnan, S. H., Jamellodin, Z. and Yusof, W. Y. W. (2015). Performance of Rice Husk Ash as a Partial Cement Replacement in Fine Grained Mortar. *Applied Mechanics and Materials*; Trans Tech Publications. Vols. 773-774, pp 969-973.
- Jamellodin, Z. Saman, H. M., Adnan, S. H., Mohammad, N. S. and Yusof, W. Y. W (2016). TFGM a New Composite material with Palm Oil Fuel Ash. Proceedings of the International Civil and Infrastructure Engineering Conference, Springer Singapore. pp 469-481
- Adnan, S. H., Yusof, W. Y. W., Jamellodin, Z., Mohammad, N. S., Saman, H. M. The Effect of Textile Fine Grained Mortar Layers on Reinforced Concrete Beam. (2016). *Jurnal Teknologi*. Penerbit UTM Press. Vol 72:1 (2016) 1–7
- Adnan, S. H., Osman, M. H., Rahman, M. A. A., Jamellodin, Z., Yusof, W. Y. W and Mohammad, N. S. (2016). Textile Fine Grained Mortar Layers on Reinforced Concrete Beam: The New Structure Technology. *AIP Conference Proceeding*. American Institute of Physics.
- Jamellodin, Z., Saman, H. M., Adnan S. H., Khalid, N. H. A., Hamid, A. A. H., Majid,
 M. A. and Salleh, N. (2018). Flexural Behaviour of Plain Concrete Prism
 Strengthened by Textile Fine Grained Mortar. Advanced Science Letters. Vol. 24, pp 3982-3985.
- Jamellodin, Z. Saman, H. M., Ibrahim, A., Adnan, S. H., Mohammad, N. S. and Khalid, N. H. A. (2019). Influence of Porosity on The Tensile Strength of Fine Grained Mortar Containing Fly Ash, Palm Oil Fuel Ash and Rice Husk Ash. *IOP Conference Series: Materials Science and Engineering*. IOP Publishing. Vol. 513.