# STRUCTURAL BEHAVIOUR OF CIRCULAR CONCRETE FILLED STEEL TUBE COLUMN FILL WITH SELF-COMPACTING CONCRETE INCORPORATING COAL BOTTOM ASH AS FINE AGGREGATE REPLACEMENT

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### **DEDICATION**

Especially to my beloved family, my supervisor and friends for giving me infinite care and blessing

Thank you for your endless support to me

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#### ABSTRACT

Lack of compaction may affect the interaction between the steel tube and the concrete, hence influence the behaviour of CFST column. Employing selfcompacting concrete (SCC) in CFST column as an infill concrete remove the necessity for compaction. However, higher volume of fine aggregate required in SCC makes SCC less preferable. The objective of this research was to investigate the behaviour of circular CFST column filled with SCC incorporating coal bottom ash (CBA) as fine aggregate replacement. In this research work, sand was replaced with CBA at 10%, 15%, 20%, 25%, and 30% of replacement levels. Tests for compressive strength, split tensile strength, flexural strength, and water absorption were performed on specimens at 7 and 28 days curing age. The testing results show that 15% of CBA was the optimum percentage suitable used in SCC. In studying the behaviour of CFST columns, 18 CFST column specimens were tested to fail under the axial compression loading. From the experimental results, it can be concluded that the utilisation of SCC with CBA in the CFST column able to improve the behaviour of CFST column. From compression testing results, the strength of CFST column filled with SCC with CBA increased about 45-50%. The experimental results were also compared with the design standard of Eurocode 4 (EC4). From the comparison, the EC4 conservatively predicts the strength of the column specimens. For instance, EC4 conservatively predicted the column strength in series I by about 21-31%. The finite element analysis (FEA) was conducted on the long column to verify the experimental results and from the FEA result, the predicted value for specimens with 140 mm diameter is 0.005% closer to the experimental results. These discoveries are significant as they showed that the utilization of CBA in SCC as an infill in CFST column able to provide the same strength as CFST with normal SCC. Besides, the use of CBA as partially replacement to the sand will help in more sustainable SCC production by lowering energy and raw material consumption.



#### ABSTRAK

Kekurangan pemadatan boleh menjejaskan interaksi antara tiub keluli dan konkrit dan secara tidak langsung mempengaruhi sifat tiang CFST. Dengan menggunakan konkrit mampat sendiri (SCC) ke dalam tiang CFST sebagai konkrit pengisi membuang keperluan untuk pemadatan. Walau bagaimanapun, jumlah agregat halus yang tinggi di dalam SCC menjadikan SCC kurang diminati. Objektif penyelidikan ini adalah untuk mengkaji sifat tiang bulat CFST yang terdiri daripada SCC yang mengunakan abu arang batu (CBA) sebagai pengganti agregat halus. Dalam kajian ini, pasir telah digantikan dengan CBA pada paras penggantian 10%, 15%, 20%, 25% dan 30%. Ujian-ujian kekuatan mampatan, kekuatan tegangan pemisahan, kekuatan lenturan dan penyerapan air dijalankan ke atas spesimen-spesimen sehingga akhir tempoh pengawetan iaitu selama 7 dan 28 hari. Hasil kajian menunjukkan bahawa 15% adalah peratusan optimum yang sesuai digunakan di dalam SCC. Dalam mengkaji sifat tiang CFST, sebanyak 18 spesimen tiang telah diuji hingga gagal fungsi apabila dibebankan dengan beban mampatan. Berdasarkan kajian, penggunaan SCC dengan CBA dalam tiang CSFT dapat mempertingkatkan sifat tiang CFST. Daripada keputusan ujian mampatan, kekuatan tiang CFST diisi dengan SCC and CBA meningkat sebanyak 45-50%. Keputusan kajian juga dibandingkan dengan standard reka bentuk Eurocode 4 (EC4). Daripada perbandingan, EC4 konservatif dalam meramal kekuatan spesimen tiang. Sebagai contoh, EC4 secara konservatif meramalkan kekuatan tiang dalam Siri I kira-kira 21-31%. Analisis unsur terhingga (FEA) juga dijalankan di tiang panjang untuk mengesahkan keputusan kajian dan FEA, Nilai ramalan spesimen dengan diameter 140 mm adalah 0.005% lebih dekat dengan hasil kajian. Penemuan ini penting kerana ia menunjukkan bahawa penggunaan CBA di dalam SCC sebagai pengisi tiang CFST dapat memberikan kekuatan yang sama seperti CFST yang di isi dengan SCC biasa. Selain daripada itu, penggunaan CBA sebagai pengganti kepada pasir akan membantu



dalam pengeluaran SCC yang lebih lestari dengan menurunkan penggunaan tenaga dan juga bahan mentah.

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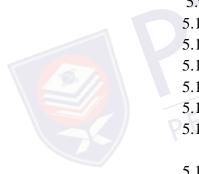
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### SYMBOLS AND ABBREVIATIONS

### Abbreviations

SCC	Self-compacting concrete
CBA	Coal bottom ash
CFST	Concrete filled steel tube
FEM	Finite Element Modeling
FEA	Finite Element Analysis
HPC	High strength concrete Superplasticiser
SP	Superplasticiser
HRWR	High range water reducer
VMA	Viscosity modifying admixture
ASTM	American society for testing and material
LOI	Loss in ignition
XRF DDUS	X-Ray Fluorescense
SEM	Scanning Electron Microscope
EGSCC	European guideline for self-compacting concrete
EFNARC	Specification and guideline for self-compacting concrete
SI	Strength Index
DI	Ductility Index
CCR	Concrete Contribution Ratio
SF	Slump Flow
LVDT	Linear Variable Differential Transducer
EC4	Eurocode 4
EC3	Eurocode 3
COV	Coefficient of variation
Symbols	

Symbols

$f_{ct}$	Tensile strength
d	Designated cross-sectional dimension
N <sub>max</sub>	Maximum load carrying capacity of CFST column
$A_s$	Cross-sectional areas of the steel
$A_c$	Cross-sectional areas of the concrete
$f_{\mathcal{Y}}$	Strength of the steel tube
f <sub>ck</sub>	Strength of the concrete
N <sub>pl,Rd</sub>	Plastic resistance of a column
N <sub>cr</sub>	Euler critical load,
EI	Effective elastic flexural stiffness
K <sub>e</sub>	Effective length factor
L	Length of CFST column
ξ	Confinement factor
ŋ <sub>a</sub>	Modification factor reflect the effect of the hoop stresses on
	the yield stress of the steel
ŋ <sub>c</sub>	Modification factor reflect the effect he effect of the tri-axial
	state of stress to the concrete strength
X	Reduction factor
$N_{Ed}$	Design value of the compression force
N <sub>c,Rd</sub>	Design cross-sectional resistance of the section to uniform
	compression force
à puS	Relative slenderness
<i>Үм</i> о	Partial safety factor
$A_{eff}$	Effective area
$N_{b,Rd}$	Buckling resistance of the compression member
t	Steel tube thickness
q	Body force
Р	Axial force
$E_s$	Elastic moduli of steel
$E_{cm}$	Elastic moduli of concrete
E <sub>r</sub>	Elastic moduli of reinforcement

# Symbols

<i>I<sub>s</sub></i> Second moment of area of structure steel se	ction
--	-------

I <sub>c</sub>	Second moment of area of concrete	
$I_r$	Second moment of area of reinforcement	
$N_{ue}$	Load capacity from experimental results	
N <sub>uc4</sub>	Load capacity predicted by Eurocode 4	
N <sub>uc3</sub>	Load capacity predicted by Eurocode 3	
	Plastic resistance of a column	
N <sub>Num</sub>	Load capacity predicted by MATLAB	

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### **CHAPTER 1**

#### **INTRODUCTION**

#### **1.1 Background of research**

The application of composite tubular columns or also known as concrete-filled steel tube (CFST) column is a structural system that offers advantages of both steel tube and concrete. The steel tube acts as permanent formwork while providing lateral confinement to the concrete whilst local buckling which commonly occurred on thin-wall steel tube are delayed by concrete (Dundu, 2012). Due to these characteristics, CFST columns have widely used in structural application as it offered higher strength and ductility, higher torsional resistance capacity, and also the ability to dissipate energy (Abed, AlHamaydeh, & Abdalla, 2013; Han, Li, & Bjorhovde, 2014) when compared to conventional reinforced concrete column and steel tube column.



In CFST column system, concrete plays an important roles in providing strength and stiffness. It not only delays local buckling and forces the steel to buckled inward, it also provide appropriate load capacity for CFST column when exposed to high temperature (Dai & Lam, 2012; Mohanraj, Kandasamy, & Rajaraman, 2010). In the past, there was numerous number of research studies were carried out on normal concrete filled in the CFST columns (Fam, Qie, & Rizkalla, 2004; Han, Liu, & Yang, 2008b; Dundu, 2012; Chang, *et al.*, 2013). Some of the literatures have been generally reviewed by Schneider (1998) and were completed for the purpose of "full" review on the literatures by Han (2002). However, a small number of research studies have been carried out on CFST columns with Self-Compacting Concrete (SCC) used as infill. This was also reported by Domone (2006) in his study. Domone (2006) analysed 11 years of case studies from 1993 to

2003 which related to SCC and the main interest of the study was to observe the distribution and the types of SCC application in the structure-based element. Out of 51 case studies, five cases involved with the use of SCC in a novel form of construction such as composite structure and thin section pre-cast units.

SCC can be defined as a flowing concrete that is able to consolidate fully under its self-weight without the tendency to segregate and bleeding. Its characteristic to flow within heavy reinforcement without the aid of vibration makes it favorable to the construction with complicated structures. Due to this characteristic, the use of SCC in the CFST column has begun to raise interest among researches. For instance, Han & Yao (2004), Tao, Han, & Wang(2005), Yang & Han (2011), Yu, Ding, & Cai, (2007), Han, Hou, & Wang (2014), Hou, Han, & Zhao (2013), and Mirmomeni, et al., (2017) studied stub columns filled with SCC with concrete cubic strength ranged from 40 to 121 MPa. From the studies, the experimental results showed that the behaviour of CFST filled with SCC have similar behaviour as composite columns filled with normal concrete. This results implying that the strength predictions used in the existing design code developed for normal concrete filled columns is suitable used for SCC filled columns within the scope of tested concrete strength. However, the ductility for very high strength SCC filled steel tubes was found to be generally smaller than that for normal strength concrete. This probably due to the brittleness of high strength concrete as reported by Qing et al., (2008) and Jamaluddin, et al., (2013). The behaviour of high strength SCC filled stub columns when exposed to standard fire are also studied by Lu, Zhao, & Han, (2009). From their study, it was found out that the behaviour of CFST column fill with high strength SCC when exposed to standard fire is almost the same as the normal concrete filled columns.

The materials used in the SCC are the same as the materials used in the production of normal concrete. This includes the use of additive material from waste by-product such as fly ash, quarry dust, and silica fume. This material was added into the concrete as part of the total constituent system. The benefits of these additive materials come from its particle size distribution characteristic and pozzolanic activity. The utilisation of combustion by-products in the SCC had gained great attention among researchers lately. This is probably due to its similarity to the fine aggregate particle size. For example, Kurniawan (2008); Lachemi, (2001); Liu,



(2010); Pathak & Siddique, (2012) investigated the potential used of pulverised fly ash in SCC as a replacement of cement. While for CBA, the study about its suitability to replace sand in the concrete and mortar was reported by Wongkeo & Chaipanich, (2010); Balasubramaniam & Thirugnanam (2015), Cheng (2012), Govindarajan *et al.*, (2014), Kim & Lee (2013) and Purushothaman & Nadu (2013). However, in the case of SCC, very limited research study can be found investigating its potential use as partial replacement material (Aswathy & Paul, 2015; Kadir, *et al.*, 2016; Kasemchaisiri & Tangtermsirikul, 2008; Siddique, Aggarwal, & Aggarwal, 2012).

Although the utilisation of SCC in the composite structure has begun to raise interest among the researchers, the use of SCC incorporating CBA as infill material is yet to be found. Apart from that, the use of the concrete incorporating CBA in the CFST column is seen able to reduce the potential of leachability of heavy metal such as arsenic in the concrete. This is due to the ability of the steel tube to confine the concrete. Owing to this advantage, the application of the CBA is suitable for this study. Therefore, the purpose of this research is to study the structural behaviour of CFST column containing SCC incorporating CBA as sand replacement material. Apart from that the use of Furthermore, a reliable finite element method (FEM) was developed to predict the bearing capacity of CFST column under axial load and then compared to the results obtained from the experimental works.



### **1.2 Problem statement**

Compaction is an important process which involved expelling the entrapped air from the concrete via concrete compactor. According to Ravindrarajah, Farrokhzadi, & Lahoud (2003), lack of compaction not only influences the permeability of the concrete, it also reduce the durability of concrete structure. In the case of composite column, the lack of compaction not only affects the properties of the core concrete itself, but also may influence the interaction between the steel tube and its core concrete, and thus influences the behaviour of the composite columns. (Han & Yao, 2003) Compaction is one of the most crucial factor that affecting bearing capacity of CFST column. In the study conducted by Han (2000) and Han & Yang (2001), it was found out that better compaction of concrete may results in higher member capacities of CFST column with circular hollow section and higher sectional capacity of CFST column for square and rectangular hollow section. However, compacting concrete mix by vibration in the concrete filled steel sections cause difficulties and may produce number of effects which impair the quality of the elements. Inserting an immersion vibrator into narrow spaces between a steel section and formwork entails the risk of vibrator coming into contact with the steel section which may leads to segregation in the bond formation area (Elzbieta & Woyciechowski, 2013). Therefore, in order to solve problem regarding compaction in composite column, the use of SCC mix is the right solution to be applied in the process of concreting composite column.

As mentioned earlier, with the promising use of SCC, many researchers have studied the potential use of SCC in the CFST column (Alwash & Al-salih, 2013; Han, Yao, & Zhao, 2005; McCann, Gardner, & Qiu, 2015; Mirmomeni *et al.*, 2017), including utilizing waste by-product in the process of making SCC mix. For instance, mineral additive such as fly ash, silica fume, and blast furnace slag was added in the SCC as the cement replacement (Han, Liu, & Yang, 2008a; Han, *et al.*, 2006; Yang, Lam, & Gardner, 2008; Qing *et al.*, 2008). However, none was found using waste by-product as replacement to the fine aggregate in the SCC as infill in the CFST column.



Regarding the SCC composition, there is no significant difference between SCC and normal vibrated concrete except for higher volume of fine aggregate and the inclusion of chemical admixture. The use of higher volume of fine aggregate in the SCC is the main concern by many as it may cause depletion on the natural resources and eventually, may lead to the environmental issues. Therefore, there is an attempt to use waste by-product in the production of SCC as fine aggregate replacement (Jiang & Mei, 2008; Johnsirani & Kumar, 2013; Kumar, Suresh, & Naidu, (2016); Patil & Gurav, (2016) including CBA (Aswathy & Paul, (2015); Kadir, *et al.*, 2016; Kasemchaisiri & Tangtermsirikul, 2008; Siddique, Aggarwal, & Aggarwal, 2012). CBA is waste by-product generated from coal-fired power plant. Unlike fly ash, which has been well accepted as pozzolanic material and commonly

used either as a component of blended Portland cement or mineral admixture in concrete (Ondova, Stevulova, & Meciarova, 2013), CBA is generally used in highway, embankment, subgrade, and subbases as reported by American Coal Ash Association (2008) or being deposited on the landfill without further use (Kadam & Patil, 2013; Kurama & Kaya, 2008; Singh & Siddique, 2014a).

In Malaysia, electricity is mainly generated from coal-fired power plant. There are seven coal-fired power plants operate daily with total capacity of 8,500 MW, which makes coal as one of Malaysia's most important sources of energy. As reported in The Star (2013), Malaysia consumes about 42 million tonnes of coal annually. The amount is devastated as it will produce tonnes of coal combustion by-products include fly ash and CBA. With future planning on the newest construction of coal-fired power plant that expected to complete in 2019 (The Star, 2017) to uphold the current demand of electricity, the amounts of coal combustion by-product are expected to increase.

As cited in Kurama & Kaya, (2008), the recycling of CBA is about 5.28% in concrete compared to fly ash with recycling rate of 47% as reported by American Coal Ash Association, with the total CBA production of about 19.8 Mtonnes in 2002. Due to the lower rate of recycling of CBA, a number of researches have investigated the potential use of CBA as partial replacement of fine aggregate in the production of concrete in terms of its strength and durability. However, the outcome from the investigation shows that the incorporation of CBA in the concrete production as a substitution of fine aggregate does not improve its compressive strength due to its porous particle structure and high water absorption (Bai, Darcy, & Basheer, (2005); Ozkan, Yuksel, & Muratoglu, (2007); Yuksel, Bilir, & Ozkan, (2007). Moreover, the waste by-product from combustion contains heavy metals within their composition and these metals are toxic to the environment as well as to human health.

According to Yahya *et al.*, (2017), the metallic element in the CBA such as copper (Cu), nickel (Ni), chromium (Cr), zinc (Zn), and lead (Pb) resulting the classification of CBA in Malaysia under the Schedule Waste (SW 104) Environment Quality Act. In the study conducted by Kadir, *et al.*, (2016), the concrete with 30% fly ash and CBA have the highest arsenic (As) leaching with 18.576 mg/L. Arsenic has gained considerable attention due to the fact that it is mobile throughout a wide



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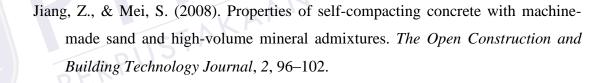
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