

mass meanwhile, elongation is calculated by separating the elongation particles and expressing their mass as a proportion of the sample's mass.

2.2.2 Asphalt Binder Properties

Asphalt constitutes 4–8% of HMA by weight and 25–30% of the cost of an HMA pavement construction, depending on the type and quantity. Paving involves the use of asphalt emulsions, asphalt cuts, and foamed asphalt. Based on the testing, softening point and penetration were investigated as well in this research by used asphalt binder grade 60/70 penetration. The softening point value is particularly essential for thick-film materials like joint and crack fillers and roofing materials by preparing a specimen exactly as specified by ASTM D36-95. Furthermore, penetration was shown to be related to viscosity, and empirical correlations for Newtonian materials have been obtained and common method for determining the consistency of an asphalt binder substance at a specific temperature.

2.2.3 Marshall Mix Design Preparation

The Marshall method of mixed design's main components is the measurement of the two important characteristics of strength and flexibility, which were bulk specific gravity determination, stability and flow test, and density and voids analysis. During the Fig. 2, HMA process, asphalt binder is combined and mixed with heated aggregate. The amount of asphalt binder in the mixture must be sufficient to ensure a layer thickness around aggregate particles and provide adequate field compaction to reduce permeability and cracking [7]. In addition, the strength of the mixture was tested in terms of 'Marshall's Stability'.



Fig. 2 Marshall mix design preparation (a) Heating material; (b) Mix material; (c) Heating loose specimen. (d) Compaction machine

2.2.4 Marshall Stability

According to Fig. 3, marshall stability is the maximum load that a specimen can withstand at a standard test temperature of 60°C. Several processes are involved, including aggregate selection, aggregate testing, asphalt binder selection, sample preparation, marshall sample compaction, marshall stability and flow testing, and result plotting. In accordance with the results, a variety of graphs might be plotted, such as density, bulk specific gravity, stability, and flow, VFA and VTM graphs [8].



Fig. 3 Machine compression test

3. Results and Discussion

The test was carried out to evaluate the performance of a hot mix asphalt mixture using limestone aggregate materials. For mixing aggregates with asphalt binder, different percentages of the aggregate total weight were employed. Sieve analysis, impact value, flakiness, and elongation, softening point, and penetration have been tested.

3.1 Material Properties

The aggregate test establishes the size, volume, and grade of aggregate required for a high-quality pavement. Sieve analysis, aggregate impact value, flakiness, and elongation have all been included. Moreover, the softening point and penetration of asphalt binder grade 60/70 penetration were examined as well in this research. The result was defined to determine the performance of an asphalt mixture. Based on Fig. 4, The grading curves for sample aggregate were consistently graded according to AASHTO T27-88 specifications.

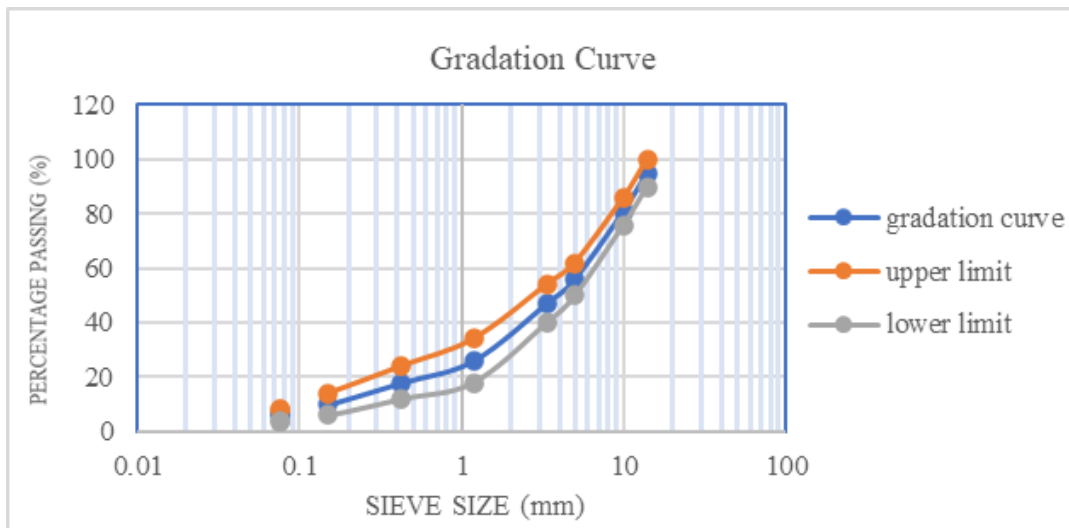


Fig. 4 Grading curve for aggregate

In Table 1, there are results were obtained for aggregate and asphalt binder properties test. The aggregate values obtained are suitable for road construction since there are in range within the 20-30% requirement for satisfactory road paving. The results for flakiness and elongation indexes were showed that there was below the limit which is 30% for JKR specification tested with two types of aggregates. Furthermore, the average softening point is 49.5°C and it’s fulfilled the required JKR specifications, it ranges from 48 to 56°C. The estimated distance for this test is 0.3 mm, and the penetration index is -0.7. This indicates that the findings of the test have been accepted due to the use of asphalt binder in conventional paving.

Table. 1 Material properties test

| Aggregate Test | | | | |
|--------------------------------|---------|---------|-----------|---------|
| Aggregate type | Granite | | Limestone | |
| Aggregate Impact Value (AIV) | 32.72% | | 26.17% | |
| Flakiness and Elongation Index | 13.596% | 20.156% | 27.394% | 17.951% |
| Asphalt Binder Test | | | | |
| Softening Point | 49.5°C | | | |
| Penetration | 6.31 mm | | | |

3.2 Volumetric Properties

The performance of hot mix asphalt mixes incorporating limestone aggregate materials has been assessed utilizing the results of a test. Marshall Mix Design performed a volumetric properties analysis on HMA comprising 0, 25, 50, 70, and 100% limestone aggregate [9].

3.2.1 Analysis on Stability

The general pattern in Fig. 5(a) shows a graph with decreasing stability from control sample to 100%. Following the compressive test, the control sample obtained the highest load for stability, 38.410 kN. The minimal load for Marshall stability was determined to be 22.242 kN for the sample 100%. This shows that the control sample has a higher stability than the other samples containing limestone aggregate. According to previous study findings, granite and limestone have more stability when applied to load, however when compared to these two aggregates, granite has greater stability than limestone [10][11]. Granite has more stability than limestone.

3.2.2 Analysis on Density

Based on Fig. 5(b), the density and sample with limestone graph was generated, revealing an increasing pattern. The sample with 70% limestone aggregate has the maximum density after compression, 2.316 g/cm³, while the control sample has the lowest density, 2.255 g/cm³. This shows that the sample containing limestone aggregate has a higher density than the sample containing granite material.

3.2.3 Analysis on Flow

According to Fig. 5(c), the flow on limestone fluctuated between 25% and 50% at 60°C, then 70% and 100%. After compression, the sample containing 100% limestone had the maximum flow value of 1.5592 mm, while the sample containing 70% had the lowest flow value of 0.3696 mm. According to the previous study finding, the asphalt binder mixture of limestone has the highest value of flow or deformation because the limestone mixture requires more asphalt binder content compared to other mixture [10][12]. This indicates that the sample containing 100% limestone had the maximum flow value compared to the sample containing 70% limestone, but did not satisfy the JKR Standard required range of 2.0 to 5.0 mm.

3.2.4 Analysis on Void in Total Mix (VTM)

The void in total mix (VTM) of a sample has been shown in Fig. 5(d), and the general pattern exhibited a bell curve. Following the compressive test, the control sample had the highest VTM value of -4.253%, while sample 50% had the lowest value of -11.397%. This shows that the control sample has a higher percentage of voids than the other samples containing limestone aggregate. The greatest value for the air void in total mix (VTM) according to the JKR specification still does not follow the standard range of 3.0 to 5.0%.

3.2.5 Analysis on Void that Filled with the Bitumen (VFB)

The void filled with bitumen (VFB) of a sample was shown in Fig. 5(e), and the general trend demonstrated a bell curve. Following the compressive test, the sample containing 70% limestone aggregate had the greatest VFB value of 93.236%, while the control sample had the lowest value of 76.367%. This shows that the 70% limestone sample has more voids than the other samples. The greatest value for the void filled with bitumen (VFB) still did not follow the standard range of 70-80%, according to the JKR specification, however the value for sample 0% was in range and followed the JKR specification standard.

3.3 Marshall Stability Test

The stiffness of a sample was shown in Fig. 5(f), with the control sample value for stiffness being 74.481 kN/mm and the optimal limestone mixture was obtained at 25% which is 69.664 kN/mm. The compressive test shown that all the sample are required followed the JKR specification for stiffness which is 2.6 kN/mm. Furthermore, this experiment shown that a mixture of asphalt and limestone additions fulfilled the requirements and can still be employed for improving the pavement.

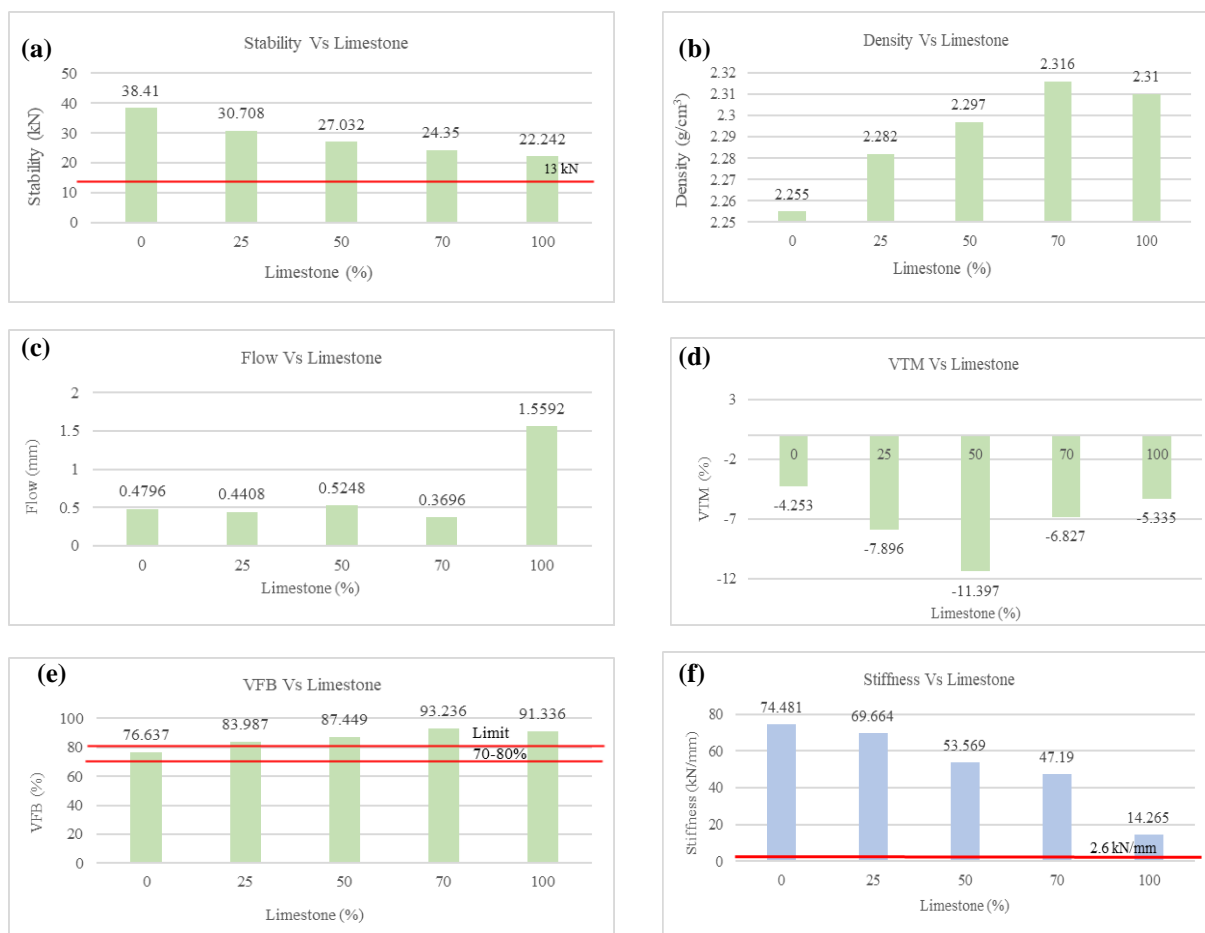


Fig. 5 Volumetric properties (a) Graph stability with limestone; (b) Graph density with limestone; (c) Graph flow with limestone; (d) Graph VTM with limestone; (e) Graph VFB with limestone; (f) Graph stiffness with limestone

4. Conclusion

From the test that have been done, it can be conclude that the stability of an asphalt sample was decreases when the amount of limestone in the mixture increases, and all the sample are still fulfilled the required for JKR specification which is the stability is higher than 13 kN. Furthermore, the incorporation of other materials into the asphalt mixture can increase the density of the sample, as control sample produced the lowest value. Moreover, the incorporation of 100% of limestone aggregate into the asphalt mixture results in a higher flow value because limestone requires higher asphalt binder content than other mixtures, the asphalt binder mixture of limestone has the highest value of flow or deformation. The void in total mix (VTM) is higher when the sample contains 50% of limestone and granite aggregate. The highest value of void filled with asphalt binder produced to 70% of limestone aggregate, and incorporating other materials into the asphalt mixture can increase the value of VFB. Other than that stiffness of an asphalt mixture decreases when the percentage of granite material decreases, but it increases when the sample containing of control sample.

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Conflict of Interest

Authors declare that there is no conflict of interests regarding the publication of the paper.

Author Contribution

