EXPERIMENTAL STUDY ON RESILIENT MODULUS OF LIQUID SILICON DIOXIDE (SIO₂) STABILIZED SUBGRADE SOIL

i

THILLAI NAYAGEE ARUMUGAM

A project report submitted in partial fulfillment of the requirement for the award of the degree of Master of Engineering (Civil –Highway Engineering)

Faculty of Civil and Environmental Engineering Universiti Tun Hussein Onn Malaysia

June 2009

Dedicated to;

To my beloved parents, Mr.Arumugam & Mrs.Saraswathy

> To my best sisters, Pathma & Keeli

To my supportive brothers and brother in law, Sugunanathan, Saravanan and Kumar

For all the love, care and support.......

ACKNOWLEDGEMENTS

With the blessing of God, I able to complete my Master in Civil Engineering successfully. First of all, I really was very grateful and would like to take this opportunity to say thank you very much to my supervisor P.M Ismail Yusof. Thank you for being my supervisor and not someone else. Thank you for all your enthusiastic guidance, numerous comments, criticisms, suggestions and insights for me during the whole 1 year of my Master Project. Not forgetting, P.M.Kemas Ahmad Zamhari, my co-supervisor who I consult and get guidance. All your support and kindness are really appreciated. With your much patience, availability and leading. I was able to complete this research, although many times you are loaded with heavy workloads.

My appreciation is also extended to all academic and non-academic members of the Faculty of Civil and Environmental Engineering, for their warm hearted cooperation in this research. My sincere appreciation also extends to all the staff of Highway Engineering Laboratory, UHTM, who provided assistance at various occasions. Their views and tips are useful indeed. Unfortunately, it is not possible to list all of them in this limited space.

I also wish to record my sincere appreciation to P.M.Haji Abd.Aziz Bin.Abd.Latiff, Prof.Dato'Haji Ismail Bin.Haji Bakar and Dr.Chan Chee Ming as the project seminar panels for providing me with comments and valuable suggestion to improve this research.

A heartfelt acknowledgement to my parents and family members for all your supports, encouragements and financial providences during my Master studies.

Last but not least, I want to express my appreciation to my fellow friend who helps me a lot in this project. Needless to say without all the above help and support, the writing and production of this project would not have been possible.



ABSTRACT

v

Village and estate roads have failed to function due to damage at earlier stage of construction and this required frequent maintenance. Weak sub-grade is the basic factor of exponential damage. In order to overcome this problem, many procedures have been developed to improve the physical behavior (strength or stiffness) of the sub grade soil. One of the procedures is to incorporate a wide range of stabilizing agent, additives or conditioners. Silicon dioxide (SiO₂) is a water based sodium silicate which is currently being patented on application by Probase Manufacturing Sdn.Bhd. to stabilize subgrade soils. The main objective of this research is to determine the resilient modulus of the liquid SiO2 stabilized sub grade soil at number of state conditions (i.e. density, moisture content and amount of stabilizer). Repeated triaxial load test was carried out at Highway Engineering Laboratory, UTHM. The specimens were prepared with maximum (100%), 90% and 95% dry density, optimum moisture content, 3% dry of optimum and 3% wet of optimum, and the amount of stabilizer was 4%, 8% and 12% of dry density of the soil. The specimens were cured for 0, 7, 14 and 28 days to determine the effect of curing days on the stabilized soil. The resilient modulus data were used to identify the best fit equation with the data. However typical pavement system analyzed using KENLAYER for non-linear elastic layer for fine grained soil; the equation applicable is the bilinear equation. Analysis of Variance (ANOVA) has been carried out to evaluate the level of significance effect of the state conditions on the stabilized soil. It has been found that density affects the resilient modulus of fine grained materials; however the magnitude of this effect is smaller compared to effect of moisture conditions. The addition of liquid SiO2 stabilizer improves the stiffness (resilient modulus) of the soil and consequently, the optimum concentration is found to be 4% for sub grade stabilization through this study.



ABSTRAK

Jalan-jalan kampung dan ladang gagal berfungsi selepas pembinaan



disebabkan oleh kerosakkan pada peringkat awal, maka penyelenggaraan kerap dilakukan bagi mengatasi masalah ini. Lapisan subgrade yang lemah menjadi punca utama berlakunya kerosakan pada jalan raya. Bagi mengatasi masalah ini, pelbagai cara telah diperkenalkan, antaranya ialah aplikasi bahan penstabil, bahan tambah dan lain-lain lagi. Silicon dioksida (SiO₂) merupakan bahan penstabil dalam bentuk cecair yang kini digunakan dalam proses penstabilan lapisan subgrade jalanraya yang diperkenalkan oleh Probase Manufacturing Sdn. Bhd. Objektif utama kajian ini ialah mengkaji modulus ketahanan tanah subgrade yang telah distabilkan dengan penstabil SiO₂ pada beberapa keadaan (i.e. ketumpatan tanah, kandungan air dan jumlah penggunaan bahan penstabil) yang berlainan. Ujian repeated triaxial load telah dijalankan di Makmal Kejuruteraan Jalanraya, UTHM. Spesimen disediakan dengan ketumpatan maksimum (100%), 95%, 90%, kandungan air optimum, 3% kurang dari optimum, 3% lebih dari optimum, dan bahan penstabil sebanyak 4%, 8% dan 12% dari berat kering tanah. Kesemua spesimen telah diawet selama 0, 7, 14 dan 28 hari untuk mengkaji kesan tempoh pengawetan terhadap tanah yang telah distabilkan dengan SiO₂. Data modulus ketahanan digunakan untuk menentukan persamaan konstitutif yang mempunyai penyuaian terbaik dengan data tersebut. Sistem jalanraya dianalisis menggunakan KENLAYER untuk lapisan kenyal yang tidak linear untuk tanah subgrade, tetapi hanya persamaan bilinear diaplikasi dalam sistem KENLAYER bagi tanah subgrade. Analisis of Variance (ANOVA) telah dijalankan bagi mengkaji kepentingan beberapa keadaan (i.e.ketumpatan tanah, kandungan air dan jumlah penggunaan bahan penstabil)atas tanah yang telah distabil. Melalui kajian ini, didapati ketumpatan tanah mempunyai kesan yang kecil terhadap tanah yang telah distabilkan jika dibandingkan dengan kandungan air. Penstabilan dengan

bahan penstabil SiO₂ meningkatkan kekerasan (modulus ketahanan) tanah subgrade. dan kepekatan optimum yang dikenalpasti melalui kajian ini ialah sebanyak 4%.

TABLE OF CONTENTS

CHAPTER

CONTENTS

PAGE

i
ii
iii
iv
V
vi
viii
xiii
xiv
xvii
xix

CHAPTER I

INTRODUCTION

1.1	Introduction	1
1.1.1	Cement Stabilization	5
1.1.2	Lime Stabilization	6
1.1.3	Other Stabilization Materials	6
1.1.4	Liquid Silicon Dioxide (SiO2) Stabilizer	7
1.2	Problem Statement	8
1.3	Objectives	11

viii

1.4	Scope of Study	11
1.5	Importance of Study	12

ix

CHAPTER II LITERATURE REVIEW

2.1	Introc	luction	14
	2.1.1	Earth Roads	15
		2.1.1.1 Loams, gravely soils and sand clay	16
		2.1.1.2 Silt soils	16
		2.1.1.3 Sands	16
		2.1.1.4 Clay soils	17
	2.1.2	Gravel roads	17
2.2	Subgi	rade	17
	2.2.1	Strength	18
	2.2.2	Moisture content	18
	2.2.3	Shrinkage and/or swelling	19
2.3	Stabil	ization	18 19 19 20
	2.3.1	Types of stabilization	20
		2.3.1.1 Mechanical stabilization	20
		2.3.1.2 Additive stabilization	20
		2.3.1.3 Modification	21
	2.3.2	Purpose of stabilization	21
	2.3.3	Characteristics of stabilization soils	22
2.4	Chem	ical Stabilization	23
PERPUS 2.4	2.4.1	Sodium Silicate Stabilization	23
	2.4.2	Lime Stabilization	24
		2.4.2.1 Cation Exchange	25
		2.4.2.2 Flocculation and agglomeration	26
	2.4.3	Impact of stabilization on	
		structural performance	26
2.5	Liqui	d Silicon Dioxide (SiO2) Stabilizer	27
2.6	Mech	anistic Empirical	28

2.7	Resili	ent Modulus	29	
	2.7.1	Definition	30	
	2.7.2	Factors affecting resilient modulus	33	
		2.7.2.1 Effect of confining pressure	33	
		2.7.2.2 Effect of deviatoric stress	34	
		2.7.2.3 Effect of Moisture Content	35	
		2.7.2.4 Effects of end conditions	37	
		2.7.2.5 Specimen size and preparation	39	
		2.7.2.6 Density and soil structure	39	
		2.7.2.7 Other factors	40	
	2.7.3	Resilient Modulus Constitutive Equation	41	
2.8	Resili	ent Modulus of lime stabilized soil	42	
2.9	Repea	ted triaxial load test	43	
	2.9.1	Testing procedures	43	
2.10	Non li	near elastic model	46	
2.11	KENI	AYER	48	
MAT	ERIAL	S AND METHODOLOGY		
2 1	Introd	uction	40	

х

CHAPTER III

MATERIALS AND METHODOLOGY

	3.1	Introduction	49
	3.2	Experimental design	52
		3.2.1 Dry density	52
		3.2.2 Moisture content	52
		3.2.3 Liquid Silicon Dioxide (SiO ₂) Stabilizer	53
DER	3.3	Number of specimens	53
F F	3.4	Analysis of variance (ANOVA)	56
:	3.5	Material	56
Î	3.6	Compaction test	57
ź	3.7	Resilient Modulus test	58
		3.7.1 Repeated triaxial load test	58
		3.7.1.1 Specimen preparation	60
		3.7.1.2 Specimen testing	64
	3.8	Analysis	65

CHAPTER IV	RESU	ULTS AND DISCUSSIONS	
	4.1	Compaction test	66
	4.2	Repeated Triaxial Load Test	68
		4.2.1 Effect of Stress State on Resilient Modulus	69
		4.2.1.1 Effect of Density	70
		4.2.1.2 Effect of Moisture	71
		4.2.1.3 Effect of Stabilizer	73
	4.3	Effect of State Conditions on Stabilized Soil	77
		4.3.1 Effect of Density	77
		4.3.2 Comparison between Treated and Untreated	I
		Based on dry density	80
		4.3.2.1 Maximum Dry Density – 100%	80
		4.3.2.2 Dry Density – 95%	81
		4.3.2.3 Dry Density – 90%	81
	4.4	Effect of Moisture Content	83
		4.4.1 Comparison between Treated and Untreated	[
		Based on Moisture Content	85
		4.4.1.1 Optimum Moisture Content	85
		4.4.1.2 Three percent (3%) Dry of Optimum	1 86
		4.4.1.3 Three percent (3%) Wet of Optimum	187
	4.5	Effect of Stabilizer	87
	4.6	Effect of Curing Days	89
	4.7	Analysis of Variance (ANOVA)	93
	4.8	Constitutive Equation	94
		4.8.1 Equation 1	95
		4.8.2 Equation 2	102
		4.8.3 Equation 3	102
		4.8.4 Equation 4	102
		4.8.5 Equation 5	109
	4.9	Analysis of Model	109

.

xi

CHAPTER V CONCLUSIONS AND RECOMMENDATIONS

5.1	Conclusions	113
5.2	Recommendations	115

хн

REFERENCES 116

ERPUSTAKAAN TUNKU TUN AMINAH

LIST OF TABLES

TABLE NO.

TITLE

PAGE

2.1	CBR Results for Probase Treated Section by IKRAM	27
2.2	Waveform and Frequency of Load	44
2.3	The Applied Stress and Number of Cycles for Fine-	
	Grained Soils	44
3.1	Number of Specimen	54
3.2	Soil Classification from Atterberg limit test	56
3.3	Testing Sequence for Subgrade Soil	64
4.1	The Value of Dry Density	67
4.2	The Value of Moisture Content	68
4.3	Analysis of Variance of the effects of state conditions	son
	Resilient Modulus	94
4.4	The Constitutive Equation	94
4.5 - 4.16	Regression Analysis for Equation 1	96 - 101
4.17 - 4.28	Regression Analysis for Equation 4	103 – 108
4.29	Results from KENLAYER using non-linear elastic	
	for subgrade layer	111

LIST OF FIGURES

FIGURES NO.

TITLE

PAGE

xiv

1.1	Types of village and estate road	2
1.2	Typical flexible pavement layers	3
2.1	Typical Pavement Cross Section	29
2.2	Representation of Resilient Modulus	31 34 46
2.3	Definition of Resilient Modulus	34
2.4	Loading form adopted in AASTHO T-307	46
2.5	Typical non-linear stress-strain relationship	46
3.1	Methodology flow chart	51
3.2	Preparation of the specimens	55
3.3	Distribution of particle size	57
3.4	Apparatus for compaction tests	58
3.5	Haversine-shaped loading waveform for	
	resilient modulus test.	59
3.6	Cylindrical Mould (100mm x 200mm)	61
3.7	Vibrator	61
3.8	Subgrade soil	61
3.9	Liquid Silicon Dioxide (SiO ₂) Stabilizer	61
3.10	Specimen	62
3.11	Vacuum	62
3.12	Specimen encased in latex membrane	62
3.13	Triaxial Chamber	62
3.14	Specimen in chamber	62

3.15	Universal Testing Machine (UTM)	63
4.1	Compaction Test Result	69
4.2	Resilient Modulus at OMC, 4% Stabilizer and 28days	
	curing.	70
4.3	Resilient Modulus at 3% Dry of Optimum, 4%	
	Stabilizer and 28 days curing	70
4.4	Resilient Modulus at 3% Wet of Optimum, 4%	
	Stabilizer and 28 days curing	71
4.5	Resilient Modulus at 90% density, 4% Stabilizer and	
	28 days curing	71
4.6	Resilient Modulus at 95% density, 4% Stabilizer and	
	28 days curing	72
4.7	Resilient Modulus at 100% density, 4% Stabilizer and	
	28 days curing	72
4.8	Resilient Modulus at 90% density, OMC and 28 days	
	curing	73 73
4.9	Resilient Modulus at 90% density, 3% Dry of Optimum	
	and 28 days curing	73
4.10	Resilient Modulus at 90% density, 3% Wet of Optimum	
	and 28 days curing	74
4.11	Resilient Modulus at 95% density, OMC and 28 days	
	curing	74
4.12	Resilient Modulus at 95% density, 3% Dry of Optimum	
	and 28 days curing	75
4.13	Resilient Modulus at 95% density, 3% Wet of Optimum	
	and 28 days curing	75
4.14	Resilient Modulus at 100% density, OMC and 28 days	
	curing	76
4.15	Resilient Modulus at 100% density, 3% Dry of Optimum	
	and 28 days curing	76
4.16	Resilient Modulus at 100% density, 3% Wet of Optimum	
	and 28 days curing	77

xv

4.17	Relationship between Resilient Modulus and Moisture	
	Content for Untreated Soil	78
4.18	Relationship between Resilient Modulus and Moisture	
	Content for Treated soil with 4% stabilizer	
	and 28 days curing	79
4.19	Relationship between Resilient Modulus and Moisture	
	Content for Untreated and Treated Soil with 100% density	80
4.20	Relationship between Resilient Modulus and Moisture	
	Content for Untreated and Treated Soil with 95% density	81
4.21	Relationship between Resilient Modulus and Moisture	
	Content for Untreated and Treated Soil with 90% density	82
4.22	Relationship between Resilient Modulus and Dry Density	
	for Untreated Soil.	83
4.23	Relationship between Resilient Modulus and Dry Density	
	with 4% Stabilizer and 28 days curing.	84
4.24 - 4.26	Relationship between Resilient Modulus and Dry Density	
	with 4% Stabilizer and 28 days curing. 85	- 87
4.27	Relationship between Resilient Modulus and Dry	
	Density with OMC	88
4.28	Relationship between Resilient Modulus and Dry	
	Density with 3% Dry of Optimum	88
4.29	Relationship between Resilient Modulus and Dry	
	Density with 3% Wet of Optimum	89
4.30	Relationship between Resilient Modulus and Curing	
	Days for 100% Density and 4% Stabilizer	91
4.31	Relationship between Resilient Modulus and Curing	
	Days for 95% Density and 4% Stabilizer	92
4.32	Relationship between Resilient Modulus and Curing	
	Days for 90% Density and 4% Stabilizer	92
4.33	Typical Pavement System for the non-linear model	110

xvi

LIST OF SYMBOLS

%	-	Percent
et al.	-	And other people
i.e.	-	In other words
UTHM	-	Universiti Tun Hussein Onn Malaysia
FKAAS		Fakulti Kejuruteraan Awam dan Alam Sekitar
km		kilometer
OMC		Optimum Moisture Content
JKR		Jabatan Kerja Raya
LVDT		linear variable differential transformer
LTPP		Long Term Pavement Protocol
UTM		Universal Testing Machine
ANOVA		Analysis of Variance
Mr		Resilient Modulus
σ_1 σ_2 PUS		major principal stress or maximum axial stress
σ_2 DIS		minor principal stress
σ3		confining pressure
σ_{d}		deviator stress
$\epsilon_{ m R}$		recoverable (resilient) axial strain
kPa		kilo Pascal
k_1, k_2, k_3 and k_4		model parameter
R ²		regression coefficient
Ра		atmospheric pressure (100kPa)
PR		Poisson Ratio
E		Elastic Modulus
٤ _v		Compressive Strain



xvii

٤t			Tensile Strain
Nd			Allowable number of load repetitions to limit
			Permanent deformation
CE	BR		California Bearing Ratio
BS	5		British Standard
Si	O_2		Silicon Dioxide
Na	1O ₂		Sodium Oxide
Na	L		Natrium
Mg	B		Magnesium
Ca	*	-	Calcium ion
K^{*}		-	Potassium ion
Al	₂ O ₃	-	Alumina
Fe	₂ O ₃	-	Iron Oxide
Na	₂ O	-	Sodium Oxide
Ti	O_2	-	Titanium Oxide
Ca	0	-	Calcium Oxide
			Titanium Oxide Calcium Oxide

xviii

LIST OF APPENDICES

Appendix A	Specimens after testing	2a
Appendix B	Effect of stress state on resilient modulus	
	On 0, 7 and 14 days of curing	3b
Appendix C	Results from Constitutive equation	12c
Appendix D	KENLAYER Results	24d
Appendix D		
Appendix E	Chemical Concentration of Liquid Silicon (SiO ₂)	
	Stabilizer	42e



CHAPTER I

INTRODUCTION

1.1 Introduction

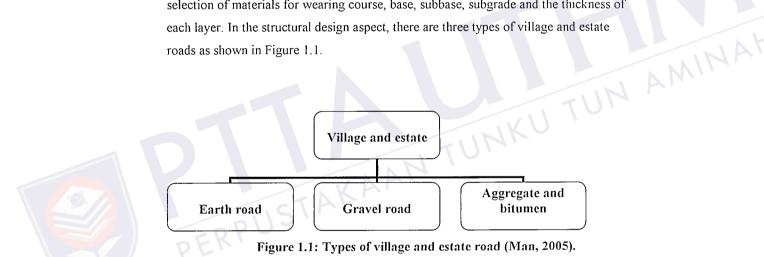


Village roads and estate roads were vital components in the national road system. This road system has functioned as the source of transportation to transfer agricultural products from the farm or estates to the urban areas, where it is processed and sold. These road networks were important transportation mode and are regarded as an essential source for the development of rural areas.

According to Yoder (1995), "in most cases, the roads are used more than its capacity. This is due to lack of systematic and comprehensive maintenance. As the damage occurs at a very early stage, it affects the functionality of the village and estate roads. The damage occurs at faster rate whenever high numbers of heavy vehicles use the road frequently. The repercussion is that the roads do not function to the maximum years it was designed to last."

One of the basic factor which results in exponential damage is weak sub-grade. This sub-grade is not properly compacted during the construction and constantly soaked in water. Water is sump on the road due to lack of drainage and lower road level compared to the shoulders. The wearing course is the surface exposed to vehicle load while bitumen functions as the adhesive. Both the wearing course and bitumen become weaker when it's constantly exposed to water. This results in deformation of the subgrade at a very early stage (Man, 2005).

The design of flexible pavement is a process where the thickness of each structural layer is determined as a structural unit which can sustain vehicular load to its maximum capacity. The most important element in the design of pavement is the selection of materials for wearing course, base, subbase, subgrade and the thickness of each layer. In the structural design aspect, there are three types of village and estate roads as shown in Figure 1.1.



Earth road does not have wearing course. Earth road is not designed. The performance of earth roads depends on the road section, characteristics of material used and type of drainage system provided. Earth road have low traffic flow, hence it is important to make sure that it is not damaged or closed, because the closure of the road will cause loss to the road users (Robert, 1998).

Gravel road is a type of road where the top of the road consists of gravel which enables the movement of vehicles particularly during rain at minimal construction cost. The gravel layer also minimizes or slows down the damage to the village road. The biggest problem with gravel road is that it has sections which deforms particularly at wheel-tracks. If the traffic volume increase, the damage to the gravel road will be faster because of the volume at the surface level will decrease once load is applied. Generally, gravel roads which experiences high traffic volume is upgraded by constructing bituminous road (JKR, 1998).

Subgrade is used to define the natural foundation or fill which directly receives the loads from the pavement. Basically most of village and estate roads consist of a subgrade layer and wearing surface layer of bitumen mixed with aggregate in accordance to the recommendation by Public Works Department (Jabatan Kerja Raya, (JKR)). Village and estate roads are classified as flexible pavement because the bitumen used in the construction of these roads is the same materials used for flexible pavement designed and build by JKR. The difference is on the number of layers only where village and estate roads have two layers i.e. sub-grade and wearing course where else other category roads have four layers i.e. sub-grade, base, sub-base and wearing course as shown in Figure 1.2.

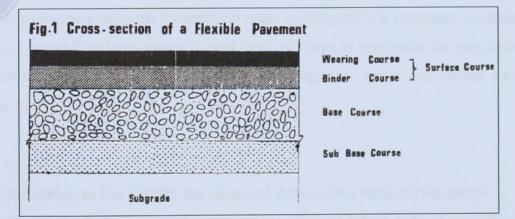


Figure 1.2: Typical flexible pavement layers (Arahan Teknik Jalan 5/85, JKR)

Flexible pavement is so called because it allows a small amount of vertical movement of the road structure under load. The main purpose of the road structure is to reduce the stress or pressure due to wheel load to a certain value that the structure can support. The intensity of the static and dynamic stress is greatest at the surface of the road and spreads in a pyramidal shape through out the depth of the structure. The load is reduced with the increasing of the load spreading. The stress is low enough for the subgrade to support it without distortion or damage (Davies, 2004).

Pavement engineers have long time recognized long term benefits in increasing the strength and durability of pavement subgrade soil by mixing additives during the reconstruction or new construction. Many procedures have been developed to improve the physical behaviour of subgrade soil by incorporating a wide range of stabilizing agents, additives or conditioners. Stabilization is a process of blending and mixing materials with a soil to improve certain properties of the soil. There are two types of stabilization that being applied in order to improve the soil and its strength, which was mechanical and chemical stabilization (Gorantla, 2005).



Stabilization without additives may be 'mechanical'- rearrangement of particles through compaction or addition or removal of soil particles. Mechanical stabilization includes compaction, and fibrous and other non-biodegradable reinforcement of geomaterials to improve strength. In applying these techniques, it is necessary to ensure the properties of stabilized geo-materials and their mixtures as applicable for use in the design of foundations, embankments, shoulders, subgrades, bases, and surface courses (Sankar, 2003).

According to Das (2002), the chemical stabilization includes the use of chemicals and emulsions as compaction aids to soils, as binders and water repellents, and as a means of modifying the behavior of clay. Chemical stabilization can aid in dust

control on roads and highways particularly on unpaved roads. It also aids in water erosion control and leaching control of waste and recycled materials.

In the selection of a stabilizer, the factors that must be considered are the type of soil to be stabilized, the purpose for which the stabilized layer will be used, the required strength and durability of the stabilized layer to support the loadings, and the cost and environmental conditions. Soil stabilizing agents such as cement, fly ash, lime, salt, or rice husk ash are used to improve the handling and engineering characteristics of soils for civil engineering purposes (Sahruzi. 2000).

1.1.1 Cement stabilization



Cement can be used either to modify and improve the quality of the soil or to transform the soil into a cemented mass with increased strength and durability. The soilcement stabilization has been used for many years because of the significant improvement in soil properties that may be achieved as a result of cement treatment. Stabilization of soils and aggregates with asphalt differs greatly from cement and lime stabilization. The basic mechanism involved in asphalt stabilization of fine-grained soils is a waterproofing phenomenon. Soil particles or soil agglomerates are coated with asphalt that prevents or slows the penetration of water which could normally result in a decrease in soil strength (Dallas et.al, 2001).

1.1.2 Lime stabilization

Generally, lime treated fine grained soils exhibit decreased plasticity which improves the strength of the soil. It also exhibits improved workability and reduced volume change characteristics of the stabilized soil. It should be emphasized that the properties of soil lime mixtures are dependent on many variables. Soil type, lime type, lime percentage and curing conditions (time, temperature, and moisture) are the most important. Stabilization occurs when the proper amount of lime is added to a reactive soil. Stabilization differs from modification in that a significant level of long-term strength gain is developed through a long-term pozzolanic reaction. This pozzolanic reaction is the formation of calcium silicate hydrates and calcium aluminate hydrates as the calcium from the lime reacts with the aluminates and silicates solubilized from the UNKU TUN AMINA clay mineral surface. This reaction can begin quickly and is responsible for some of the effects of modification (soil stabilization for pavement) (Eddie, 2004).

Other stabilization material 1.1.3



The sodium ions present in salt stabilization react with clay particles, giving greater dispersions of some clay. This makes it possible to obtain desired compaction with less effort. It provides a strong bond between soil particles. After curing and recrystallization of the un-reacted salt within the void has taken place, a firm, stable layer is formed, increasing the strength and load bearing capacity. Salt has considerable use in stabilizing the surface of dirt roads with low traffic volume (Eddie, 2004).

REFERENCES

- AASHTO (1993). "AASHTO Guide for Design of Pavement Structures." American Association of State Highway and Transportation Officials: Washington, D.C.
- AASHTO T 307 (Modified) (2009). "Determining the Resilient Modulus of Soils and Aggregate Materials." Washington State Department of Transportation.
- AASHTO (2004). "Standard Specifications for Transportation Materials and Methods of Sampling and Testing (24th edt.)." American Association of State Highway and Transportation Officials: Washington, D.C.
- Abdalla Basel A. (2003). "Material Properties for Implementation of Mechanistic Empirical (M-E) Pavement Design Procedures in OHIO." College of Engineering and Technology, Ohio University.

Abushoglin and F.; Khogali, W. (2006). "Resilient Modulus and Permanent Deformation Test for Unbound Materials." National Research: Council Canada.



Alexandre B. Parreira and Ricardo F. Goncalves. "The Influence of Moisture Content and Soil Suction on the Resilient Modulus of a Lateritic Subgrade Soil." University of Sao Paulo: Sao Carlos, Brazil.

Ali Maher, Thomas Bennert and Dr. Nenad Gucunski. (2000). "Resilient Modulus Properties Of New Jersey Subgrade Soils." Department of Transportation, the State of New Jersey.

Al-Refeai.T and A.Al-Suhaibani. (2000). "Factor Affecting Resilient Behavior Of Subgrade Soils In Saudi Arabia." College of Engineering, Saudi Arabia.

- Anuroopa Khancherla, (2004). "Resilient Modulus and Permanent Deformation Testing of Unbound Granular Materials." Texas A&M University
- Austroads Pavement Research Group. (1993). "Characterisation of Unbound Pavement and Subgrade Materials Using Repeated Loading Triaxial Testing." Australian Road Research Board Ltd.APRG Report No.8.
- Baystate Roads Program. (1995). "Stabilizing Road Soil." Department of Civil and Environment Engineering, University of Massachusetts.
- Davies Beresford Obafemi Arnold (2004). "A Model for the Prediction of Subgrade Soil Resilient Modulus for Flexible-Pavement Design: Influence of Moisture Content and Climate Change." The University of Toledo.
- Daehyeon Kim and Nayyar Zia . (2002) "Simplification of Resilient Modulus Testing for Subgrades". Proposal for Research and Implementation Study, Geotechnical Engineer INDOT Research Division and INDOT Materials and Tests Division, Indiana Department of Transportation, November 5.



Daehyeon Kim and Nayyar Zia. (2002). "Simplification of Resilient Modulus Testing for Subgrades." Indiana Department of Transportation, Research Division, West Lafayette, Indiana.

Daehyeon Kim and Nayyar Zia Siddiki (2006). "Simplification of Resilient Modulus Testing for Subgrades." Indiana Department of Transportation.

Dallas N. Little and F. A. M. Shafee yYusuf. (2001). "Example Problem Illustrating the Application of the National Lime Association Mixture Design and Testing Protocol (MDTP) to Ascertain Engineering Properties of Lime-treated Subgrades for Mechanistic Pavement Design/Analysis."

- Dallas N. Little, Eric H. Males, Jan R. Prusinski, and Barry Stewart. (2001) "Cementitious Stabilization." Louisiana State University.
- Dallas N. Little, (1999). "Evaluation of Structural Properties of Lime Stabilized Soils and Aggregates." National Lime Association.
- Dallas N. Little, (2000). "Evaluation of Structural Properties of Lime Stabilized Soils and Aggregates." National Lime Association.
- Dallas N. Little, Thompson, M. R., Terrel, R. L., Epps, J. A., and Barenberg, E. J.,
 (1987), "Soil Stabilization for Roadways and Airfields," Report ESL-TR-86-19,
 Air Force Services and Engineering Center, Tyndall Air Force Base, Florida.
- Das, B.M. (2002). "Advanced Soil Mechanics." 2nd ed. London: Spon Press. 125-138.
- David L. Bennett, (1996). "Effect of Subbase on Modulus of Subgrade Reaction." U.S. Department of Transportation.

Department of the Army, The Navy, and The Air Force.(1994) "Soil Stabilizations for Pavements." Air Force Afjman.

Design Procedures. (2008). "Soil Modification or Stabilization" Office of Geotechnical Engineering, Indiana.

Dong-Gyou Kim, M. S, (2004) "Development of a constitutive model for resilient modulus of cohesive soils". Ohio State University.

Drumm, E. C., Boateng-Poku, Y. and Pierce, T. J (1990), "Estimation of Subgrade Resilient Modulus from Standard Tests". Journal of Geotechnical and Geoenvironmental Engineering, ASCE.



- Eddie Y. Chou. (2004). "Structural Support of Lime or Cement Stabilized Subgrade Used with Flexible Pavements." Ohio Department of Transportation, Columbus.
- Harrigan Edward T. (2004). "Laboratory Determination of Resilient Modulus for Flexible Pavement Design." Research Readers Digest: National Cooperative Highway Research Program.
- Federal Highway Administration. (2007). "Geotechnical Aspects of Pavements Reference Manual." U.S.Department of Transportation.

Final Report. (2004). "Guide for Mechanistic-Empirical Design – Chapter 2: Material Characterization." West University Avenue.

Fredrick Lekarp, Ulf Isacsson, and Andrew Dawson. (2000) "Resilient Response of Unbound Aggregates." Journal of Transportation Engineering.

George Vorobieff and Greg Murphy. (2003) "A New Approach to Pavement Design Using Lime Stabilized Subgrade." Australia Stabilization Industry Association and Pavement Technology Ltd.

George K.P. (2004). "Prediction of Resilient Modulus from Soil Index Properties." The University Of Mississippi, Mississippi.

Gorantla Vasu Babu.(2002) "Review on Recent Advances in Ground Improvement Techniques." 2nd International Conference on Advances in Soft Soil Engineering and Technology 2-4th July 2003. Putrajaya, Malaysia.

Hani H. Titi, Mohammed B. Elias, and Sam Helwany (2006). "Determination of Typical Resilient Modulus Values for Selected Soils in Wisconsin." University of Wisconsin: Wisconsin Highway Research:

- Huang Y.H. (2004). "Pavement Analysis and Design." 2nd Edition. Unites States, Prentice-Hall, pp 90-290.
- Inge Hoff, Leif J. Bakløkk and Joralf Aurstad (1998). "Influence of Laboratory Compaction Method on Unbound Granular Materials." Roads and Transport, Trondheim, Norway.
- Jagannath Mallela, Harold Von Quintus, P.E. and Kelly L. Smith. (2004). "Consideration of Lime Stabilized Layers In Mechanistic Empirical Pavement Design." University Avenue, Champaign.
- Jason Lowe. (2007). "Evaluation of the Repeat Load Triaxial Test and its Potential for Classifying Base course Aggregates." The IOQ and AQA NZ Annual Combined Conference.

Jayalecthumy Satee. (1990) "Effect of Curing Period on Sodium Silicate Stabilized Soil". Faculty of Civil Engineering, University Technology of Malaysia.

Jean-Louis Briaud and Spencer J.Buchanan. (2000). "Introduction to Soil Moduli." Professor Texas A&M University.

Jha J.N and Gill K.S (2006). "Effect of Rice Husk on Lime Stabilization." Engineering College, Ludhiana (Punjab).

JKR. (1998). "Manual on Pavement Design." Jabatan Kerja Raya, Kuala Lumpur, Malaysia.

JKR. (1998) "Standard Specification for Road Works." Jabatan Kerja Raya, Kuala Lumpur.



- Jorge A. Prozzi and Samer M. Madanat. "A Non-Linear Model for Predicting Pavement Serviceability." University of California, Berkeley.
- Junhwan Lee, Jihwan Kim, and Beongjoon Kang (2009). "Normalized Resilient Modulus Model for Subbase and Subgrade Based on Stress dependent Modulus Degradation." Journal of Transportation Engineering.
- Leena Korkiala-Tanttu. (2008). "Calculation Method for Permanent Deformation of Unbound Pavement Materials." Helsinki University of Technology, Finland.

Liquid Sodium Silicates. (2008). "Sodium Silicate." United States National Library of Medicine.

Mahmud Kamil El.Rayes and Ahmed El.Tayeb Ahmed. (2002) "Lime Soil Stabilization." National Building Research Station, University of Khartoum, Sudan.

Man Nyuk Chaw. (2005), "Maintenance of Road in Kota Marudu." Degree of Bachelor of Civil Engineering, Universiti Teknologi Malaysia.

Mark Morvant P.E. (2004), "Evaluation of Subgrade Stabilization on Pavement Performance." Louisiana Transportation Research Center.

Mark Popik, M.Eng., P.Eng and Chris Olidis, P.Eng. (2005) "The Effect of Seasonal Variations on the Resilient Modulus of Unbound Materials." Annual Conference of the Transportation Association of Canada Calgary, Alberta.

Michael A. Habte and Nasser Khalili. (2008). "A Constitutive Model for Elastoviscoplastic Behaviour of Pavement Materials." University of New South Wales, Sydney. Australia.

- Mohammad, L. N., Titi, H. H. and Herath, A. (1998), "Intrusion Technology: An Innovative Approach to Evaluate Resilient Modulus of Subgrade Soils", Geotechnial Special Publication.
- Mohd Hizam bin Harun. (2005). "The New JKR Manual on Pavement Design." Jabatan Kerja Raya, Kuala Lumpur.
- Molenaar A.A.A (2008). "Repeated Load CBR Testing, a Simple But Effective Tool For The Characterization of Fine Soils and Unbound Materials." Delft University of Technology, Netherlands.
- Morched Zeghal, Yassin E. Adam, Osman Ali and Elhussein H. Mohamed. Elhussein H. Mohamed. (2005). "Review of the New Mechanistic-Empirical Pavement Design Guide-A Material Characterization Perspective." Annual Conference of the Transportation Association of Canada, Calgary, Alberta
- Naji, Khoury N., Zaman, Musharraf M., Nevels, James B. and Mann, Jerry.(2003)
 "Effect of Soil Suction on Resilient Modulus of Subgrade Soil Using the Filter Paper Technique." University of Oklahoma.



National Lime Association (2004). Lime-treated Soil Construction Manual "Lime Stabilization & Lime Modification."

Neringa Verveckaite, Jonas Amsiejus, and Vincentas Stragys. (2006). "Stress-Strain Analysis In The Soil Sample During Laboratory Testing." Dept of Geotechnical Engineering, Vilnius Gediminas Technical University.

Neubauer, C. H., and Thompson, M. R., (1972), "Stability Properties of Uncured Lime-Treated Fine Grained Soils," In Highway Research Record 381, HRB, National Research Council, Washington, D.C., pp. 20-26.

- Ng Pui Ling (2005) "Determination of Optimum Concentration of Lime Solution for Soil Stabilization". Faculty of Civil Engineering, Universiti Teknologi Malaysia.
- Ni.B, T.C.Hopkins, L.Sun and T.L.Beckham. (2002). "Modelling the Resilient Modulus of Soils." University of Kentucky, Transportation Center, Lexington, USA.
- Norman D.D and Jr.Kyle Bennett (2005). "Development of Testing Protocol and Correlations for Resilient Modulus of Subgrade Soils." Final Report: University of Arkansas.
- Olidis Chris, P.Eng. and David Hein, P.Eng.(2005) "Guide for the Mechanistic-Empirical Design of New and Rehabilitated Pavement Structures." Applied Research Associates Inc. - ERES Consultants Division.
- Ravindra Gudishala. (2004). "Development of Resilient Modulus Prediction Models for Base and Subgrade Pavement Layers From in Situ Devices Test Results." Sri Krishna Devaraya University, Andhra Pradesh, India.

Research Report. (1995). "Resilient Modulus of Kentucky Soil." Kentucky Transportation Center, University of Kentucky.

Richard P.L. (1998). "Resilient Modulus of Subgrades." Graduate Assistant: University of Connecticut.

Richter Cheryl Allen. (2006). "Seasonal Variations in the Moduli of Unbound Pavement Layers." Federal Highway Administration, McLean, VA 22101.

Robert Mokwa, P.E. and Michelle Akin (2001). "Measurement and Evaluation of Subgrade Soil." Montana Department of Transportation, Helena, Montana.



- Robert P.E, D.D.Norman and Yanjun Qiu (1998). "Permanent Deformation of Subgrade Soils." University of Arkansas.
- Peter Davich, Joseph F. Labuz, Bojan Guzina and Andrew Drescher. (2004). "Small Strain and Resilient Modulus Testing of Granular Soils." Department of Civil Engineering University of Minnesota.
- Pierre Hornych and Absamad El. Abd, LCPC. (2004). "Selection and Evaluation of Models for Prediction of Permanent Deformations of Unbound Granular Materials in Road Pavements." Sustainable and Advanced MAterials for Road InfraStructure (SAMARIS).
- Protocol P46. (1996). "Resilient Modulus of Unbound Granular Base/Subbase Materials and Subgrade Soils." U.S.Department of Transportation.
- Prozzi Jorge A and Madanat Samer M (2001). "A non-linear model for Predicting Pavement Serviceability." University of California, Berkeley.

Puppala, A. J., Mohammad and Allen, A., (1996) "Engineering Behavior of Lime-Treated Louisiana Subgrade Soil," Transportation Research Record No. 1548.

Sahruzi Sahari.(2000) "Sub-Grade Stabilization Using Lime". Managing Director, Stabilized Pavements (Malaysia) Sdn.Bhd.

Saleh M & S.J.JI. (2000). "Factors Affecting Resilient Modulus," University of Canterbury, Christchurch, New Zealand.

Sankar Bhattacharja and Javed I. Bhatty. (2003) "Comparative Performance of Portland Cement and Lime Stabilization of Moderate to High Plasticity." Portland Cement Association.



- Santa, B. L. (1994), "Resilient Modulus of Subgrade Soils: Comparison of Two Constitutive Equations". Transportation Research Record, 1462, TRB, National Research Council, Washington, D.C., pp. 79 - 90.
- Saravut Jaritngam and Sommart Swasdi (2007). "Improvement for Soft Soil by Soil-Cement Mixing." Soft Soil Engineering. 2. 637-641.

Seminar (2008). "Soil Subgrade Stabilization." Kuala Lumpur.

- Şenol, A, Bin-Shafique, Edil and Benson. (2002). "Use of Class C Fly Ash For Stabilization of Soft Subgrade." Department of Civil and Environmental Engineering University of Wisconsin-Madison, USA.
- Shahar Husin, Ismail Saad and Mohd.Sabri Hasim. "The Benefit of Soil Subgrade Stabilization Using Cement for Asphalt Pavement In Malaysia". Highway Innovative (Hitec) Sdn.Bhd.and Reka Analisis Pakar Ingenieurs (RAPI) Sdn.Bhd.

Sha Xian A. (1997). "Strength and Deformation Principles of Stabilized Soil." Highway University, China.

Shongtao Dai and John Zollars. (1786). "Resilient Modulus of Minnesota Road Research Project Subgrade Soil." Minnesota Department of Transportation, Maplewood.

Shongtao Dai. (2001) "Resilient Modulus of Minnesota Road Subgrade Soil." Minnesota Department of Transportation: Gervais Avenue.

Stan Vitton. (1995). "Introduction to Soil Stabilization." Michigan Technological University.



- Thomas Smiley S., Ph.D.(1999). "Soil Density Analysis." Technical Report, Bartlett Tree Research Laboratories.
- Technical Buletin. (2002) "Lime for Modification and Stabilization." Natural Chemicals.
- Technical Note. (2001) "Repeated Load Triaxial Test for Unbound Pavement Materials." Geopave.
- Thompson, M. R., and Robnett, Q. L. (1979), "Resilient Properties of Subgrade Soils". Proceedings of ASCE, 105(TE1)
- Torsten Mayrberger and Ralph J. Hodek. (2007). "Resilient Modulus at the Limits of Gradation and Varying Degrees of Saturation." University Transportation Centre.
- Wilson, B. E., Sargand, S. M., Hazen, G. A., and Green, R. (1990). "Multiaxial Testing of Subgrade", Transportation Research Record, 1278, TRB, National Research Council, Washington, D.C.

Yi Bian. (2003) "Subgrade Under the Pavement." Term Project for ECI281A.

Yoder E.J. and M.W.Witczak. (1995). "Principles of Pavement Design." A Wiley-Intersience Publication, New York.

